

# A CUSTOM DESIGN JERICHO

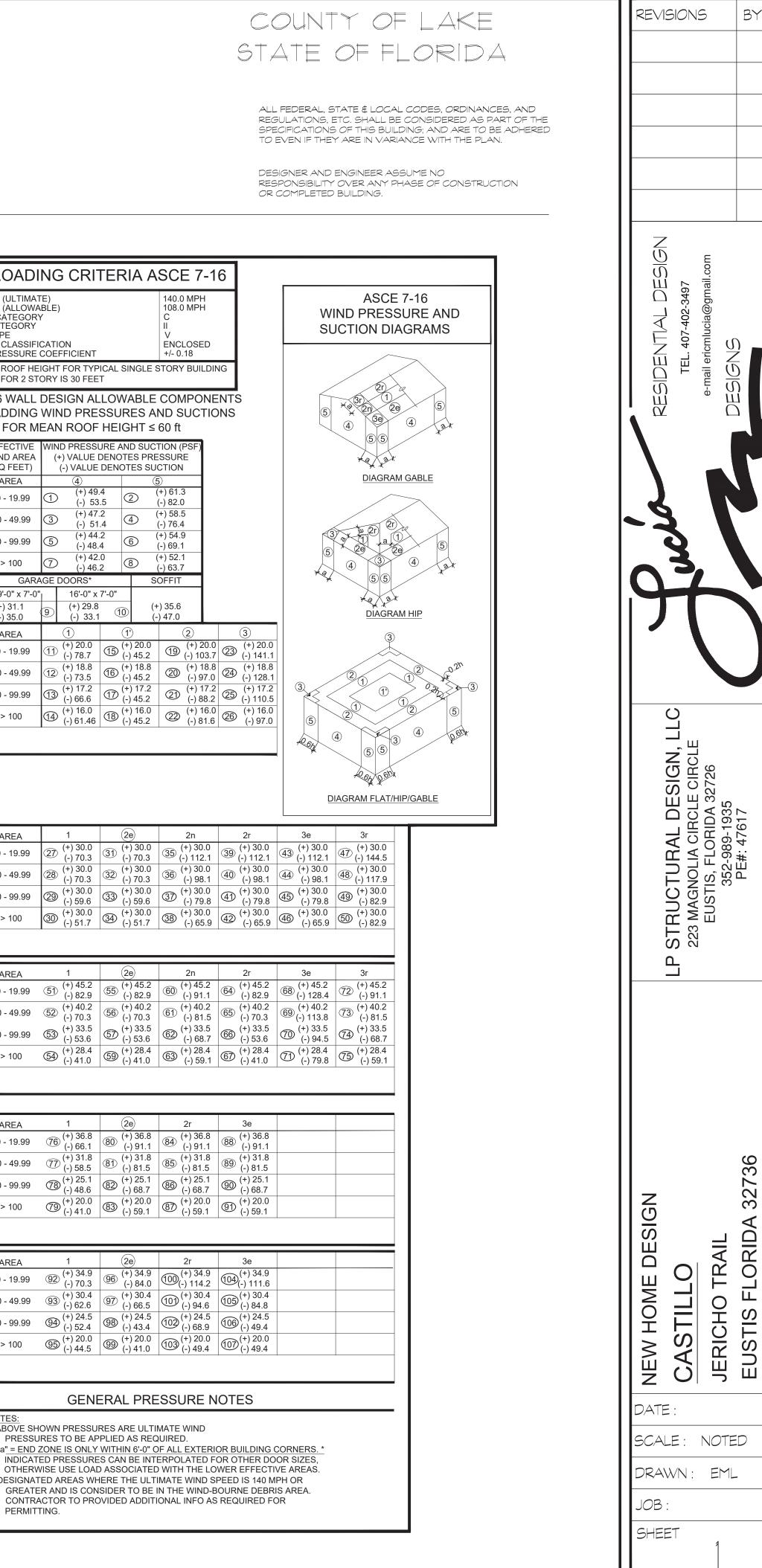
## STRUCTURAL DESIGN CRITERIA

URAL NOTES	
CONCRETE	CODE CRITERIA WIND SPEED (ULTIM
STRENGTH AT 28 DAYS OF 3000 PSI (SLABS) 3000 PSI (COLUMNS AND BEAMS), A SLUMP OF 5" (ENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.63 G OF ALL TOP BARS OF BEAMS. ND CORNERS OR CORNER BARS WITH A 25" LAP PROVIDED EA WAY. OR 1 1/2" TO FORM U.N.O. 4A / A1064M. WWF SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE ON GRADE TO BE MIN 1.5 LBS OF FIBER PER CUBIC YARD 3E NEW DOMESTIC DEFORMED BARS FREE FROM RUST, SCALE & OIL & SHALL MEET ASTM A615/ SHALL BE SUPPORTED ON PRE-CAST CONCRETE PADS, STEEL WIRE OR PLASTIC SUPPORT. TOP TEMPORARY STRINGERS. DOWELS FOR COLUMNS & FILLED CELLS SHALL BE SECURED IN TIED TO FOOTING REINFORCING. SPLICES IN REINFORCING WHERE PERMITTED SHALL BE AS	<ul> <li>FLORIDA BUILDING CODE 7TH EDITION (2020) RESIDENTIAL.</li> <li>FLORIDA FIRE PREVENTION CODE 7TH EDITION (2020)</li> <li>FLORIDA BUILDING CODE ACCESSIBILITY 7TH EDITION (2020)</li> <li>FLORIDA BUILDING CODE ACCESSIBILITY 7TH EDITION (2020)</li> <li>NFPA 70-14. NATIONAL ELECTRICAL CODES. (NEC 2017) &amp; 6TH FBCR CH. 34-43</li> <li>BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE - (ACI 318-14).</li> <li>SPECIFICATIONS FOR STRUCTURAL CONCRETE - (ACI 301-10).</li> <li>BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES - (ACI 530-13).</li> <li>NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION - 2015 EDITION.</li> </ul>
N THE DESIGN OF THIS PRODUCT. IF CONTRACTORS WISH TO USE A DIFFERENT EPOXY, THEY DR WRITTEN APPROVAL. I GAS PREVALENT AREAS, APPENDIX "F" OF THE FLORIDA BUILDING CODE 7TH EDITION (2020) IS IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I. THEREFORE, ANY AND ALL NOTES ON EPLACED WITH 3000 P.S.I. FOR THE CONCRETE STRENGTH.	<ul> <li>WOOD FRAMED CONSTRUCTION MANUAL 2015 EDITION.</li> <li>APA PLYWOOD DESIGN SPECIFICATION 2012 EDITION.</li> <li>AMERICAN SOCIETY OF CIVIL ENGINEERS: ASCE/SEI 7-16</li> <li>ALUMINUM DESIGN MANUAL - 2015 EDITION</li> </ul>
GHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90-014, WITH A MINIMUM NET COMPRESSIVE	の 20 - 49.9
270-12A. A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS Y INSPECTIONS ARE REQUIRED DURING CONSTRUCTION E AS NOTED ON THE DRAWINGS WITH THE CELLS FILLED WITH COARSE GROUT. HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 DIA OR 10FT ED IN THE CENTER OF THE MASONRY CELL WITH MIN 1/2" CLEARANCE TO INSIDE FACE.	TTX 20-43.9 50-99.9 > 100
S05/L1, UNLESS OTHERWISE NOTED ON THE DRAWINGS. M. PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE	GENERAL ROOF LOADING
T PAPER AS A STOP IS PROHIBITED. VIDE STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR S SHALL BE ABOVE AND BELOW ALL WALL OPENINGS	SHINGLE ROOF (PSF)METAL ROOF (PSF)TILE ROOF (PSF)HEAVY (+) 31.1 (-) 35.0
FOR (3) DAYS AND NO CONCENTRATED LOADS FOR (7) DAYS. PER CODE ACI 318-14 IECHANICAL VIBRATION, AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL OUT SHALL BE FLUSH WITH TOP OF WALL.	TOP CHORD LL TOP CHORD DL         20 10         20 10         20 10         20 15         20 25         20 25         AREA           BOTTOM CHORD LL* BOTTOM CHORD DL         0         0         0         0         0         10         10         10         10         10         10         10         10         10         10         10         10         20 - 49.9           TOTAL (PSF)         40         40         45         55         55         10 - 19.9 <td< td=""></td<>
SHEAR WALLS, AND MISC. STRUCTURAL WOOD FRAMING MEMBERS,(I.E. BLOCKING OR GABLE AN OR IN DETAILS. IF CONFLICTS OCCUR BETWEEN PLAN AND DETAILS, THE STRONGEST STRUCTURAL FRAMING MEMBERS SHALL BE SPF #2. FOR DRY USE ONLY (MOISTURE CONTENT 19% OR LESS), U.N.O. ALL WATERPROOFING AND THE CONTRACTOR AND ARE TO BE DESIGNED AND DETAILED BY OTHERS IAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION	TOTAL (PSF)       40       40       45       55       III III       20-49.9         BOTTOM CHORD LL (OPT) ATTICS W/ LIMITED STORAGE       20       IIII III       50-99.9         ATTICS W/ HEAVY STORAGE       50       IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII
NES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES,TYP., U.N.O. CHEMICALS THAT ARE CORROSIVE TO STEEL. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CT APPROPRIATE CONNECTORS THAT RESIST CORROSION. FOR EXAMPLE, ACQ-C, ACQ-D, STAINLESS STEEL FASTENERS. DOT SODIUM BORATE (SBX) DOES NOT.	APPROVAL FROM EOR OR INDICATED ON PLAN GENERAL FLOOR LOADING
TH OR CONCRETE TO BE PRESSURE TREATED. WITH CONCRETE OR MASONRY. SEAT PLATES SHALL BE PROVIDED AT BEARING LOCATIONS	TOP CHORD LL40 (PSF)COMMENTS:TOP CHORD DL10 (PSF)BOTTOM CHORD LL0 (PSF)
IIN VALUES U.N.O.	BOTTOM CHORD DL 5 (PSF)
400 PSI) MIN. WALL AND FLOOR SHEATHING REQUIREMENTS ALONG W/ NAILING INFORMATION OTHERWISE:	SPECIAL FLOOR LOADING       0         GAME ROOM / READING ROOMS       60 (PSF)       COMMENTS:         BALCONIES / DECKS       40(PSF)       d. A SINGLE CONCENTRATED LOAD         A SINGLE CONCENTRATED LOAD       0       20 - 49.9
18/24) SHEATHING SHALL FINISH FLUSH TO EXTERIOR WALL FACE. URE 1 OR 132" RATED OSB EXPOSURE 1. A MINIMUM?" SPACE IS RECOMMENDED BETWEEN OR EXPANSION. PER R604.3 SHEATHING SHALL NOT BE USED AS WEATHER RESISTANCE BARRIER	BALCONIES OVER 100 SQ:FT       100(PSF)       APPLIED IN ANY DIRECTION AT ANY       20 40.0         LIGHT STORAGE       125(PSF)       POINT ALONG THE TOP.       00       00         GUARDRAILS AND HANDRAILS       200(LBS)(d)       f. BALUSTERS AND PANELS FILLERS       50 - 99.9         GUARDRAIL IN-FILL COMPONENTS 50 (LBS)(f)       SHALL BE DESIGNED TO WITHSTAND       100       100         STAIRS / NON SLEEPING ROOMS       40 (PSF)       A HORIZONTALLY APPLIED NORMAL       100       100         SLEEPING ROOMS       30 (PSF)       LOAD OF 50 POUNDS ON AN AREA       100       100       100
ISS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF ORS APPLIED. PLEASE COORDINATE THE TRUSS ENGINEER FOR THE LOCATION OF THESE	DEFLECTION CRITERIA       0 0         ROOF TRUSSES*       LL/360       TL/240         ROOF RAFTERS       LL/360       TL/240         ROOF RAFTERS (W/O CLG)       LL/360       TL/240         FLOOR TRUSSES/ BEAMS **       LL/360       TL/240         FLOOR I-JOIST***       LL/480       TL/240
	*TL MAX 2" UP TO 40FT SPAN ****TL MAX 1/4" DIFFERENTIAL BETWEEN **TL MAX 3/4" ADJACENT TRUSSES ADJACENT TRUSSES ADJACENT TRUSSES
CTIONS: ASTM A992, GRADE 50, Fy=50 KSI TUBE STEEL (HSS): ASTM A500, GRADE B, Fy = 46 KSI PIPE STEEL RUCTURAL & MISC. STEEL: A36 Fy=36 KSI STRUCTURAL CONNECTIONS: ALL STRUCTURAL BOLTS TO BE A TO BE A307 THREADED ROD SHALL CONFORM TO A36 OR A307 ANCHOR BOLTS SHALL CONFORM TO ASTM M A-307 SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL REINFORCEMENT SHOP DRAWINGS TO BE F ON FOR REVIEW AND APPROVAL	A325N U.N.O M F1554 ALL
AL BOLTS TO BE A325N U.N.O. ALL A325N BOLTS SHALL BE BROUGHT TO A "SNUG-TIGHT" CONDITION , AS I .TS MUST BE FULLY TENSIONED PER SPECIFICATION STRUCTURAL BOLTS SMALLER THAN 5/8" DIA. TO BE R A307 ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 ALL BOLTS CAST IN CONCRETE: ASTM A36 OR A	E A307 SHEET INDEX
ES STEEL REINFORCEMENT SHOP DRAWINGS TO BE PROVIDED TO ENGINEER OF RECORD BEFORE FABRI IONS: ELECTRODES - E70XX UNO (LOW HYDROGEN). FILLET WELDS SHALL BE 3∕16" UNO.	1 COVER SHEET STRUCTURAL 10 - 19.9

- 1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS PER 2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. 3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND 4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS. 5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE
- 6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION. 7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES . SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS, AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 3 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. 8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL
- 1. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED WITH 1/2" DIA. EPOXY ANCHORS WITH 7" EMBEDMENT. SIMPSON "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURER'S RECOMMENDATIONS OR SIMPSON 1/2" TITEN HD BOLTS WITH MINIMUM 7" EMBEDMENT. SEE PLAN FOR EMBEDMENT 2. FOR MISSED VERT. DOWELS, DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY (SIMPSON HIGH STRENGTH EPOXY-TIE ANCHORING ADHESIVE ) MIXED PER THE MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO THE MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM 3. FOR MORTAR JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT ( BAR DOES NOT HAVE TO BE CONT. TO FOOTING ). 4. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) 1/2"x 21/2" TITENS TO MASONRY AND (7)-10d NAILS TO TRUSS FOR UPLIFTS LESS THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1720#). IF CORNER STRAP IS MISSED, CONTRACTOR IS TO INSTALL (2) SIMPSON HGAM10 W/ (4) 1/4" x 1 1/2" SDS SCREWS AND (5) 1/4" x 2 1/4" TITENS ONE EACH SIDE OF TRUSS. 5. NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN 3 IN A ROW WITHOUT APPROVAL FROM EOR. IF GIRDER TRUSS CONNECTIONS ARE MISSED, 6. IF MISSED, MSTAM36 OR MSTAM40 STRAP IS MISSED FOR 2ND FLOOR JAMB STUD CONNECTION, CONTRACTOR MAY INSTALL SIMPSON HTT5 W/ (26) 16d x 21/2" NAILS AND 5/8" ANCHOR BOLT SET IN SIMPSON HIGH STRENGTH EPOXY W/ MIN 6" EMBEDMENT AND MIN 3" EDGE DISTANCE. CONTACT EOR IF STRAPS ARE

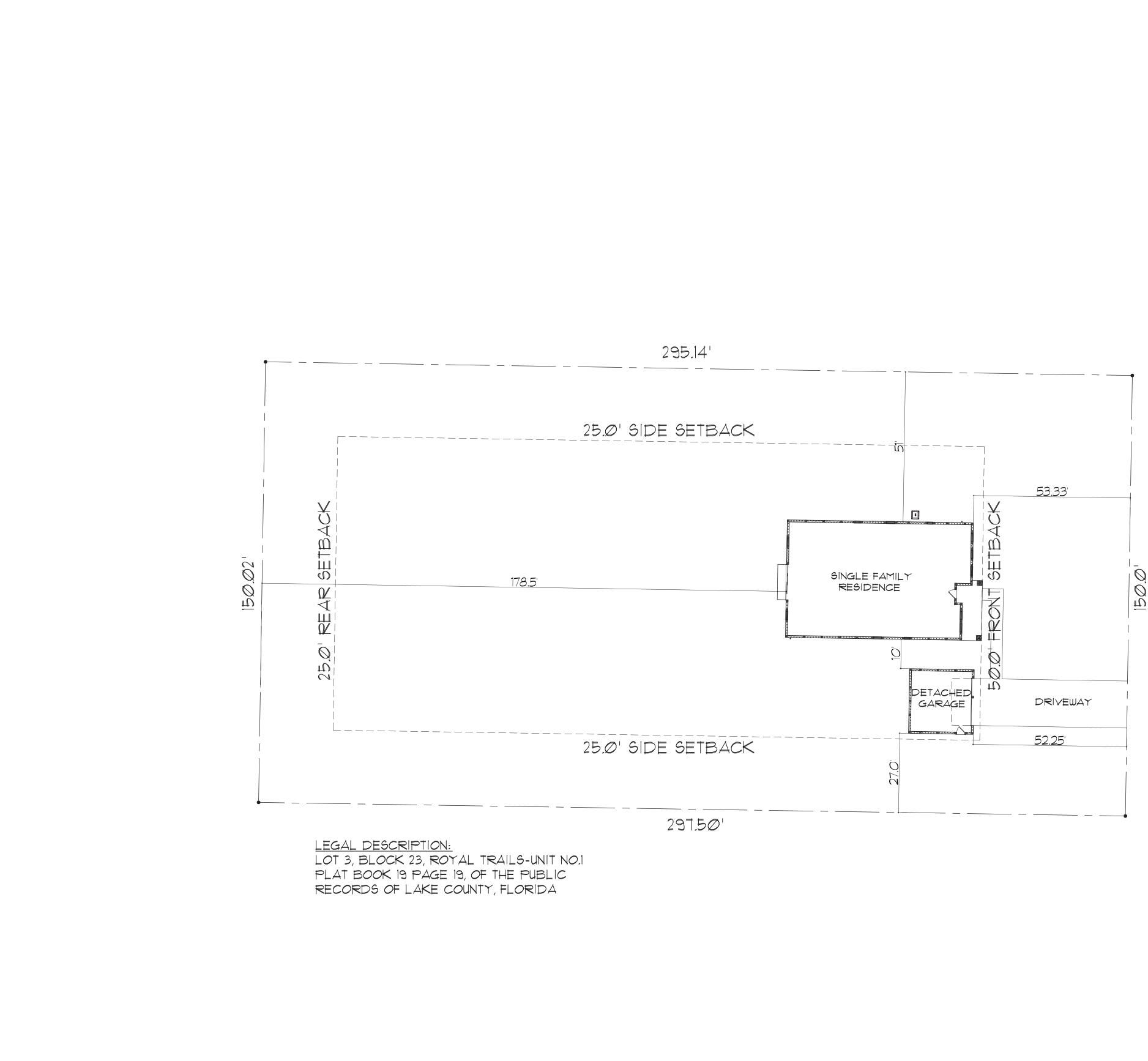
	SHEET INDEX
1	COVER SHEET STRUCTURAL NOTES, CODE COMPLIANCE, SPECS AND WIND PRESSURES
2	SITE PLAN
3	FLOOR PLAN - NOTED
4	FLOOR -PLAN - DIMENSIONED
5	EXTERIOR ELEVATIONS
6	ROOF FRAMING PLAN
7	DETAILS
8	ELECTRICAL PLAN
9	FOUNDATION PLAN
10	FOOTING DETAILS
11	LINTEL LOADING TABLES AND CONCRETE DETAILS
12	CONNECTOR SCHEDULE AND ENGINEERING DETAILS
13	HEADER SCHEDULE AND ENGINEERING DETAILS

	FOR M
	EFFECTIVE WIND AREA (SQ FEET)
	AREA 10 - 19.99
LS	20 - 49.99
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2	50 - 99.99
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SHEETS

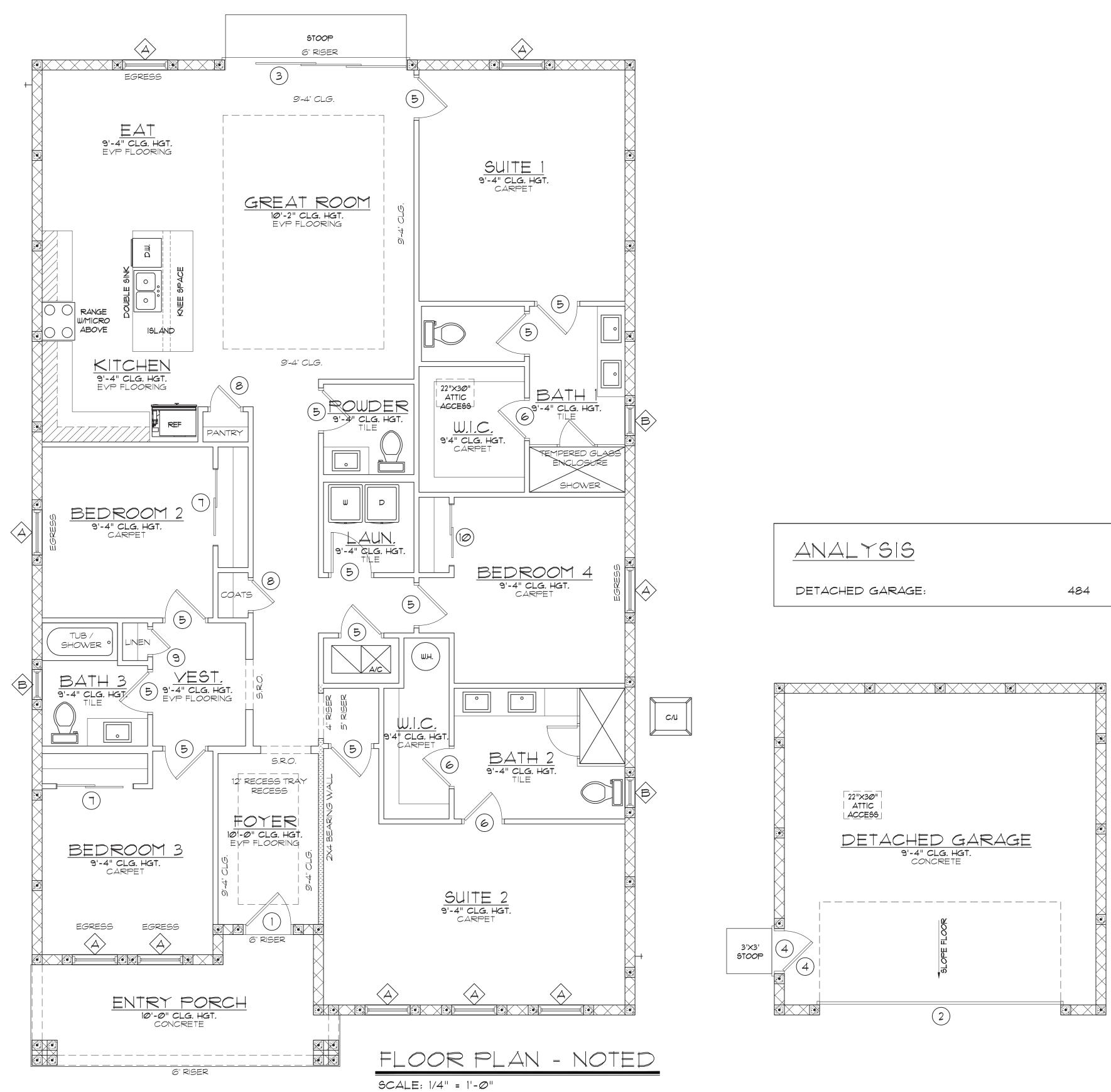


	RESIDENTIAL DESIGN TEL. 407-402-3497 e-mail ericritucia@gmail.com
JERICHO TRAIL	LP STRUCTURAL DESIGN, LLC 223 MAGNOLIA CIRCLE CIRCLE 223 MAGNOLIA CIRCLE CIRCLE 2352-989-1935 752-989-1935 75376
	DATE : CASTILLO DERICHO TRAIL JCBTIS FLORIDA 32736 CASTILLO DEVENDA 32736 JOB :
SITE PLAN Scale: 1"=20.0'	SHEET 2
	OF 13 SHEETS

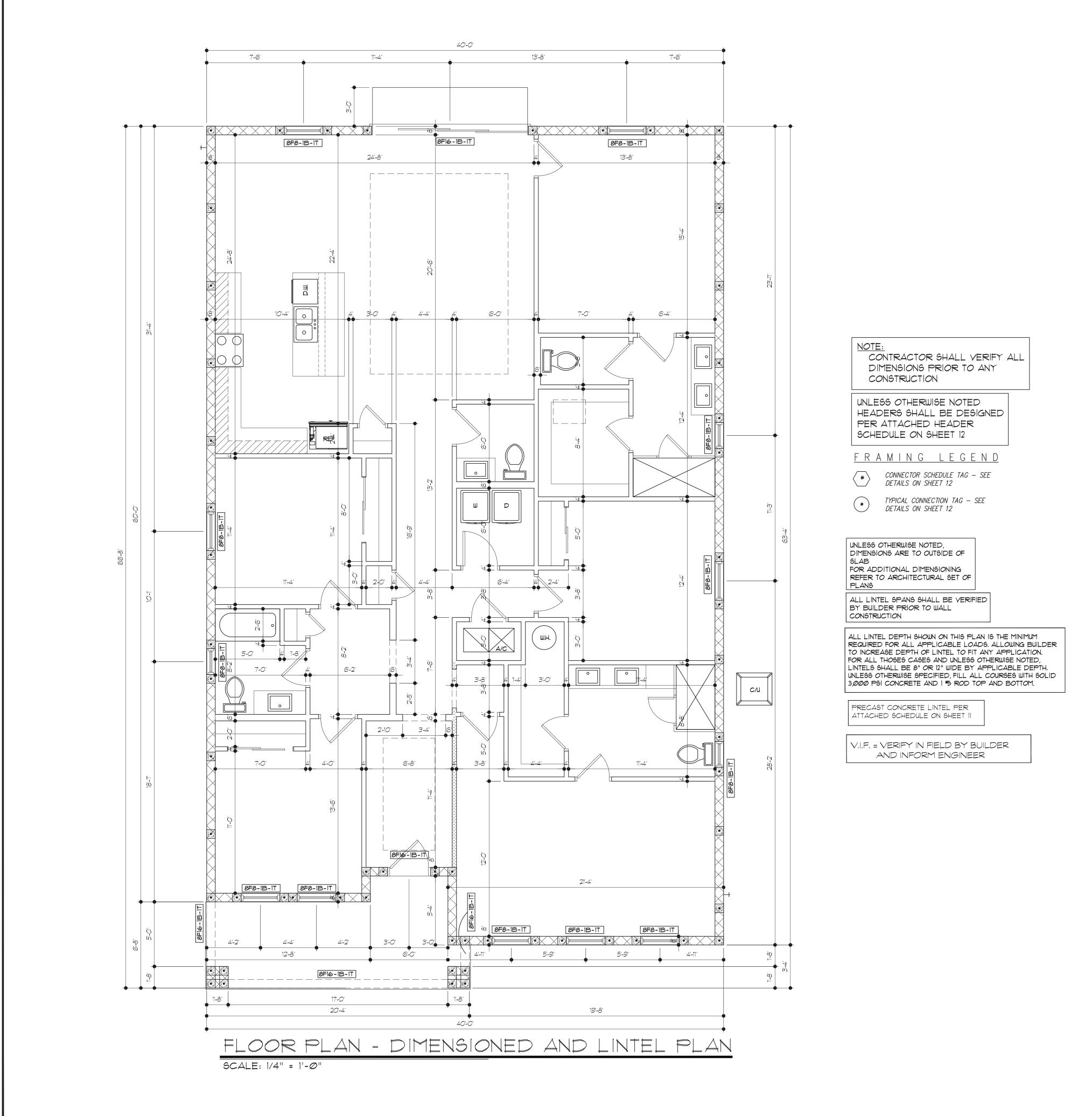
## WINDOW SCHEDULE NOTES TAG WIDTH HGT. QTY. CASEMENT 3'-Ø" 5'-Ø" $\langle A \rangle$ 9 B CASEMENT - TEMPERED 2'-Ø" 4'-Ø" 3 DOOR SCHEDULE EXTERIOR TAG WIDTH HGT. QTY. NOTES 3'-Ø" 8'-Ø" FULL GLASS (1)(2)16'-0" 8'-0" OVERHEAD GARAGE DOOR 3-PANEL SLIDING GLASS (3) 12'-Ø" 8'-Ø" (4)EXTERIOR GARAGE SWING 2'-8" 8'-Ø" INTERIOR TAG WIDTH HGT. QTY. NOTES (5) 2'-8" 8'-Ø" SWING SWING 6 2'-6" 8'-Ø" (T) 5'-0" 8'-0" BI-PASS 2 2'-Ø" 8'-Ø" SWING (8) (9) 1'-8" 8'-Ø" SWING (10)4'-0" 8'-0" BI-PASS ANALYSIS 2459 LIVING: FRONT PORCH: 142

TOTAL UNDER ROOF:

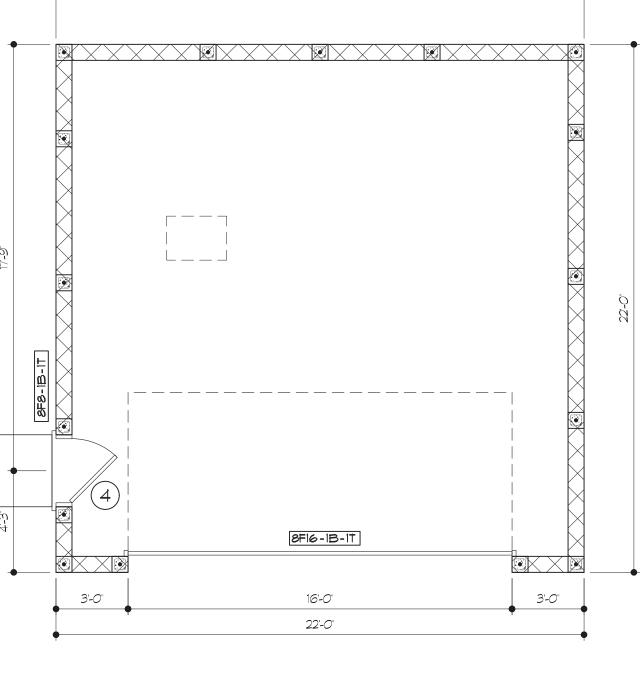
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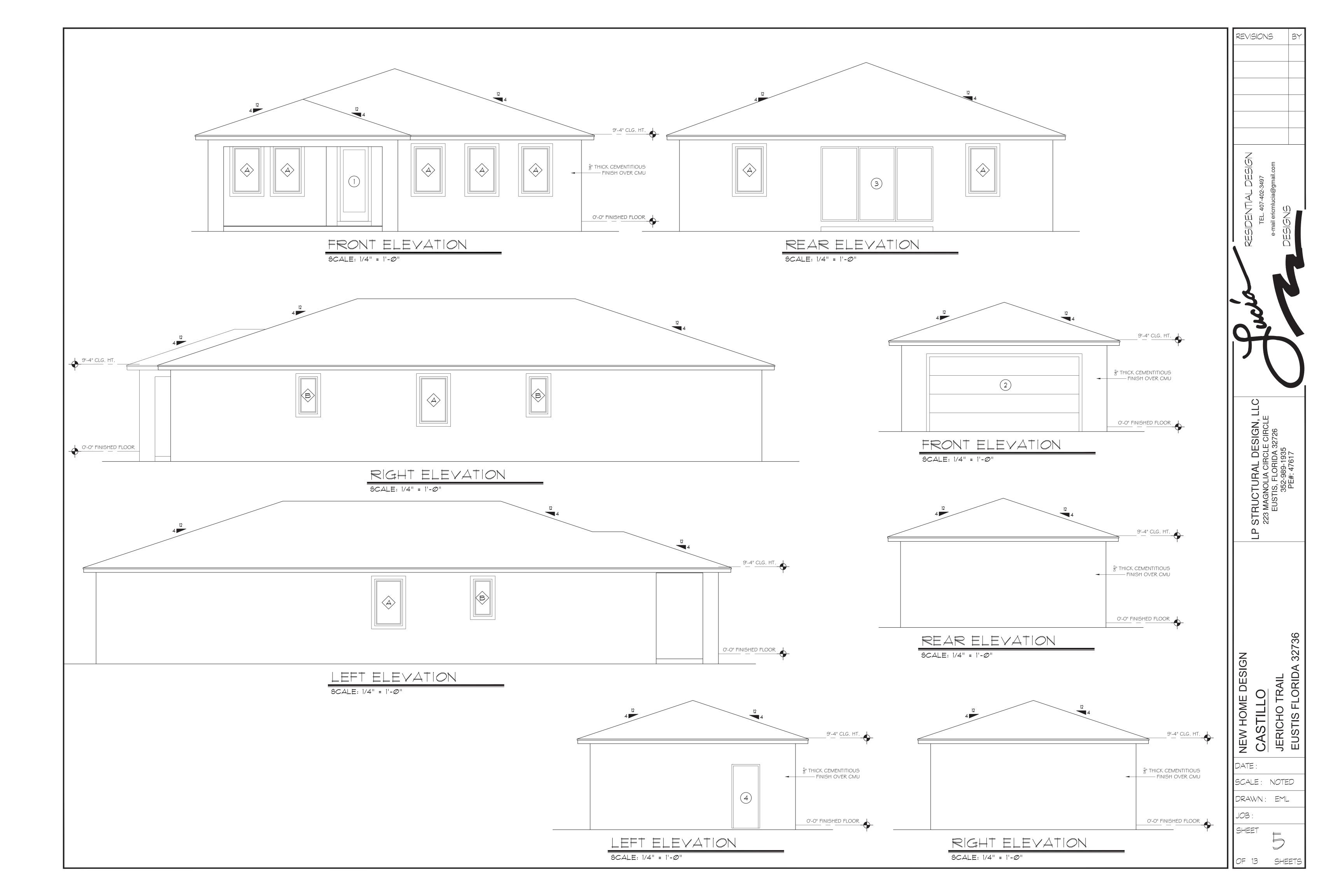
REVISIONS BY	/
RESIDENTIAL DESIGN TEL. 407-402-3497 e-mail ericmlucia@gmail.com DESIGNS	
LP STRUCTURAL DESIGN, LLC 223 MAGNOLIA CIRCLE CIRCLE 223 MAGNOLIA CIRCLE CIRCLE EUSTIS, FLORIDA 32726 352-989-1935 PE#: 47617	
LP STRUCTU 223 MAGNOI 223 MAGNOI EUSTIS, 352 PE	
NEW HOME DESIGN CASTILLO JERICHO TRAIL EUSTIS FLORIDA 32736	
DATE: SCALE: NOTED DRAWN: EML JOB: SHEET 3	

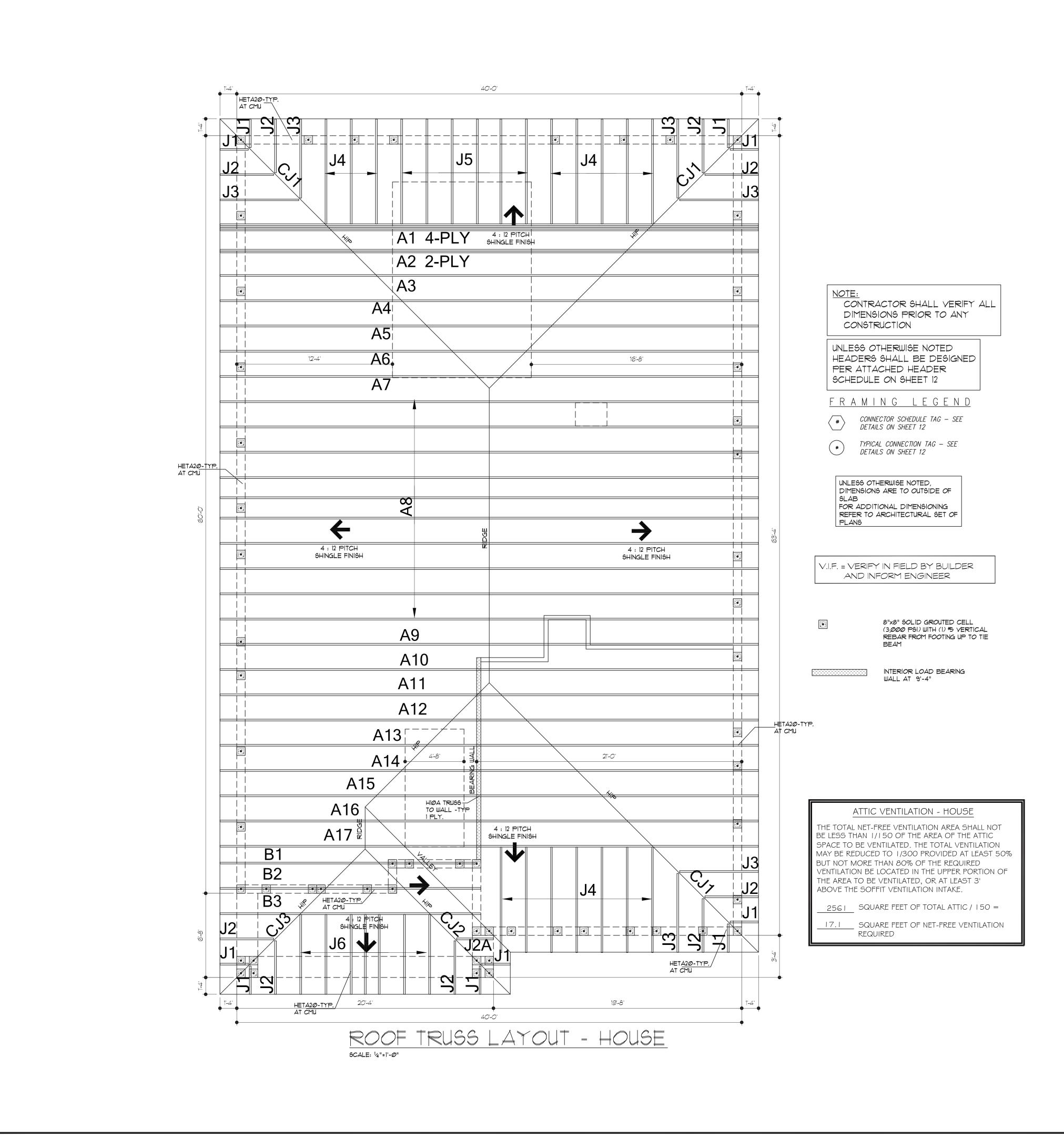


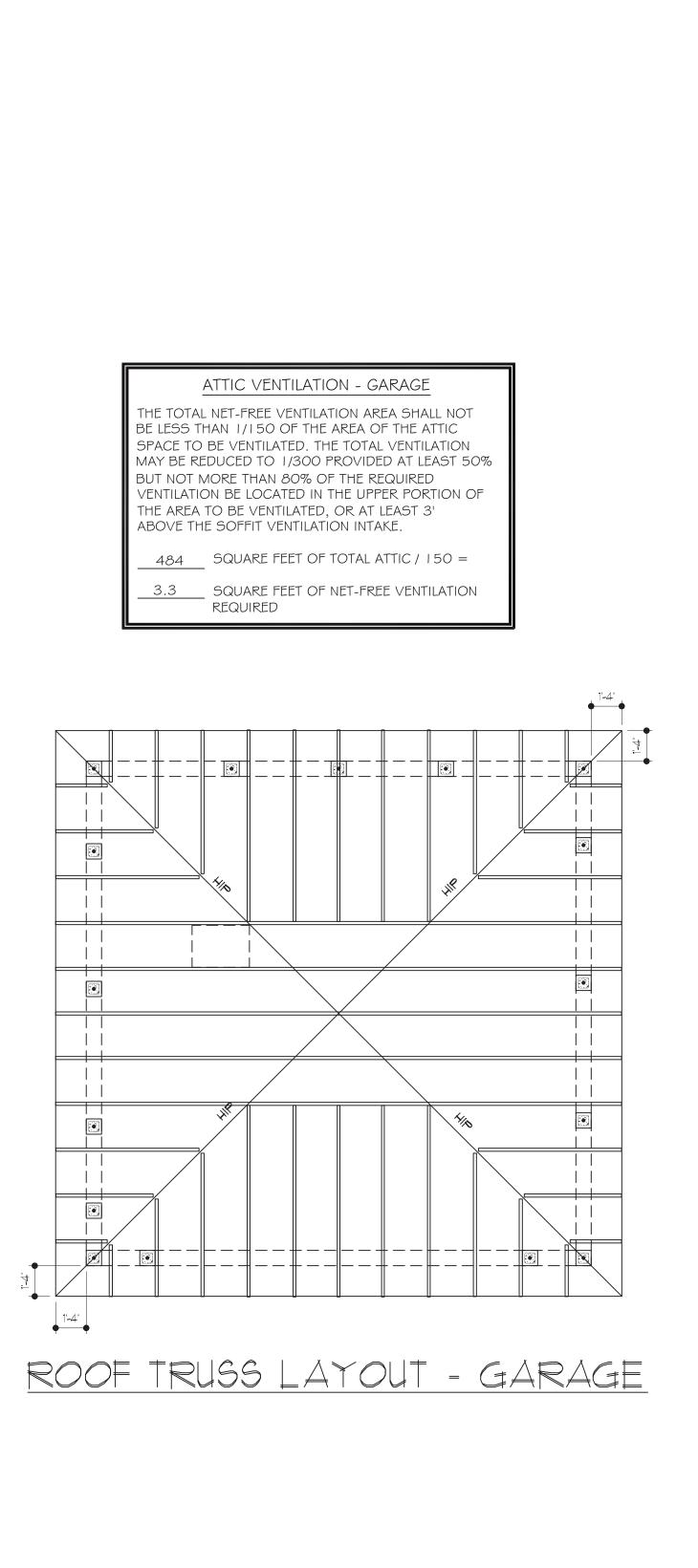


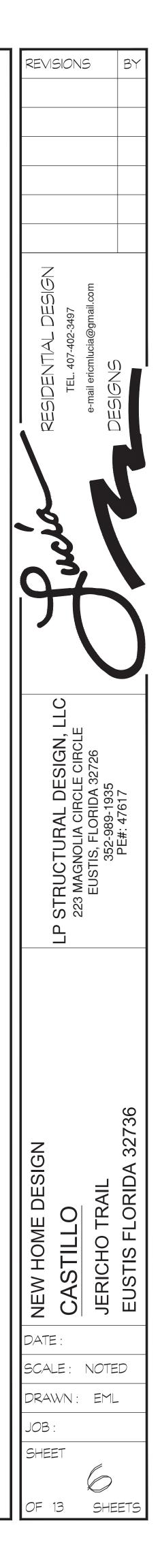


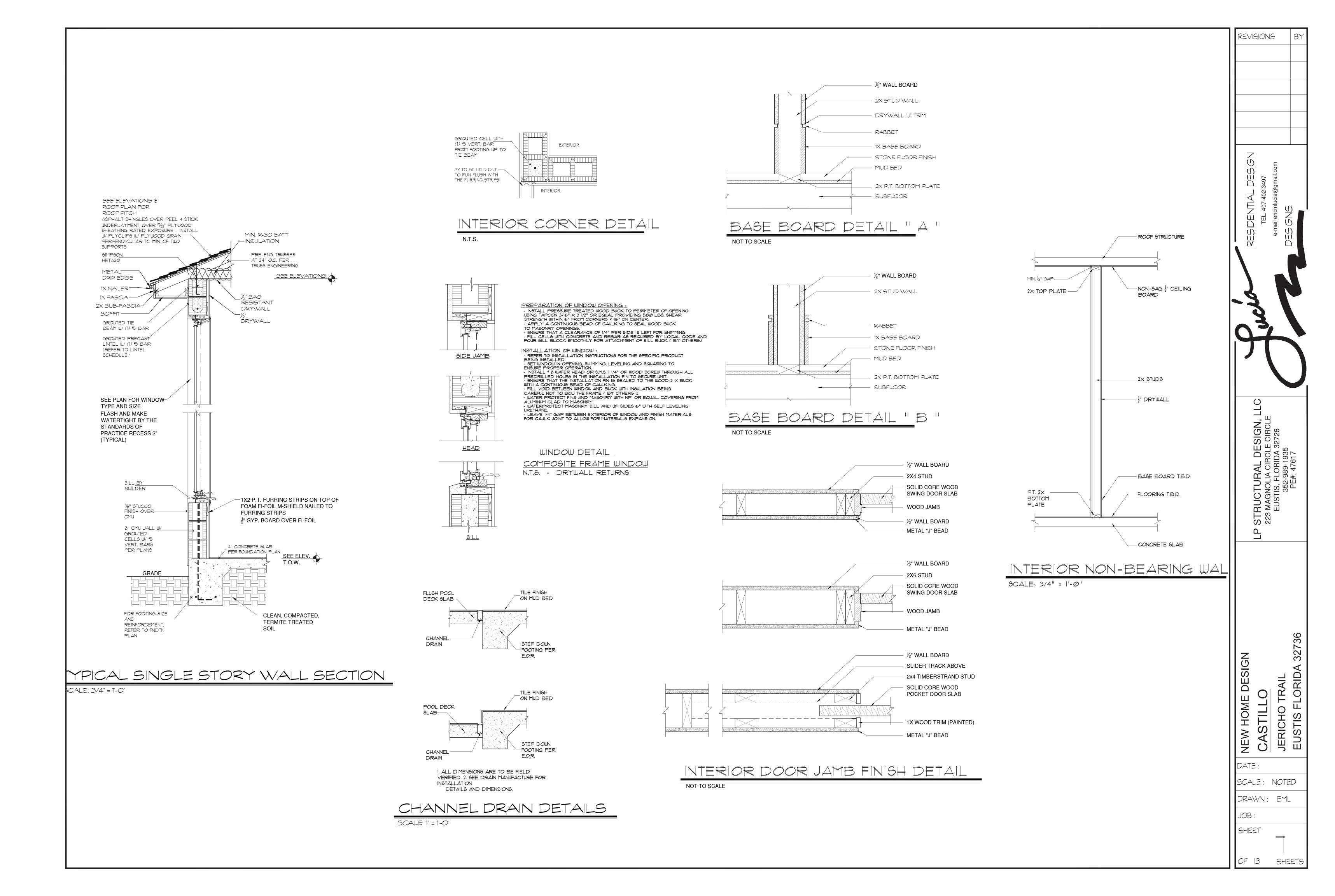
22'-0"



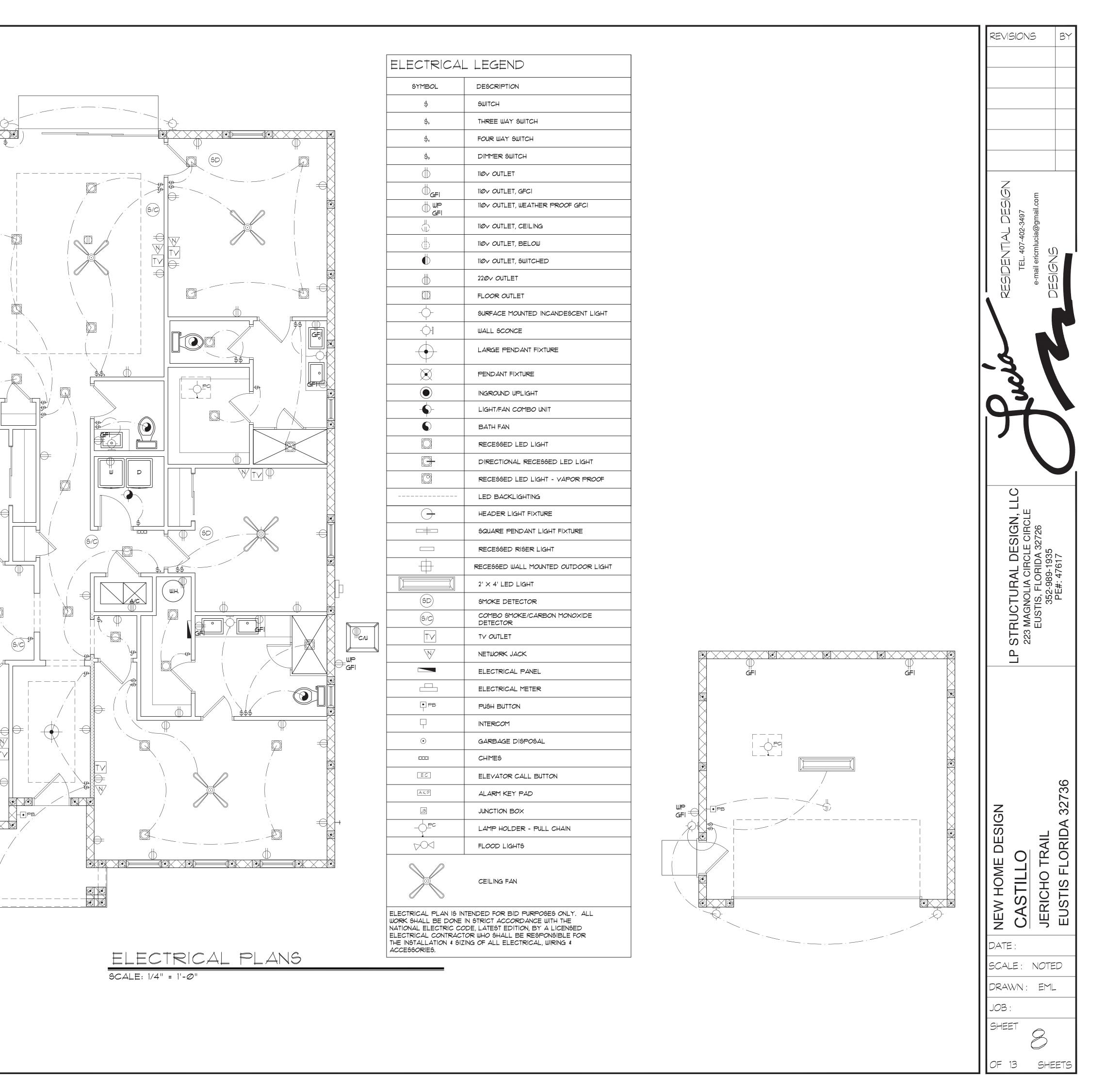


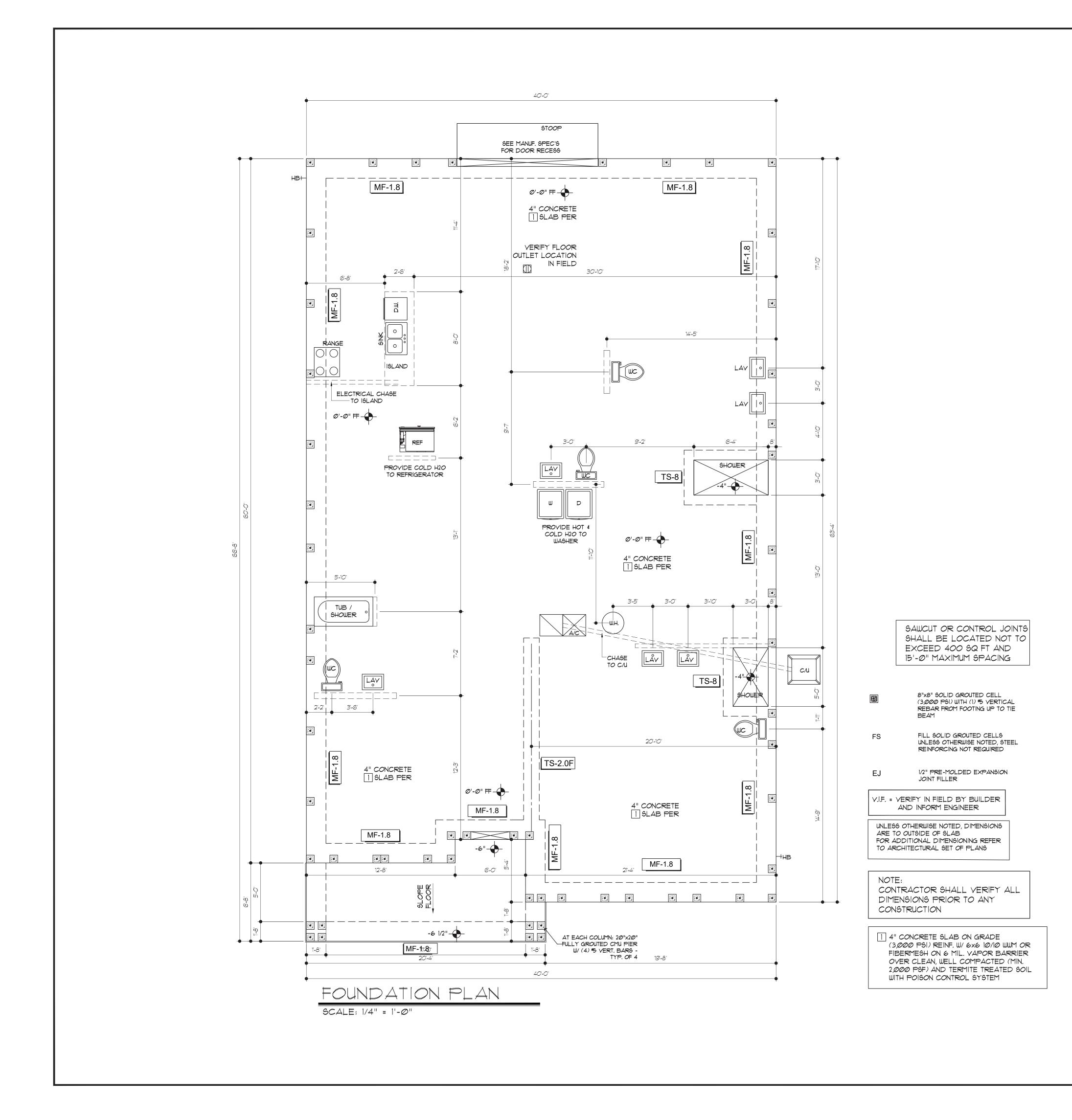


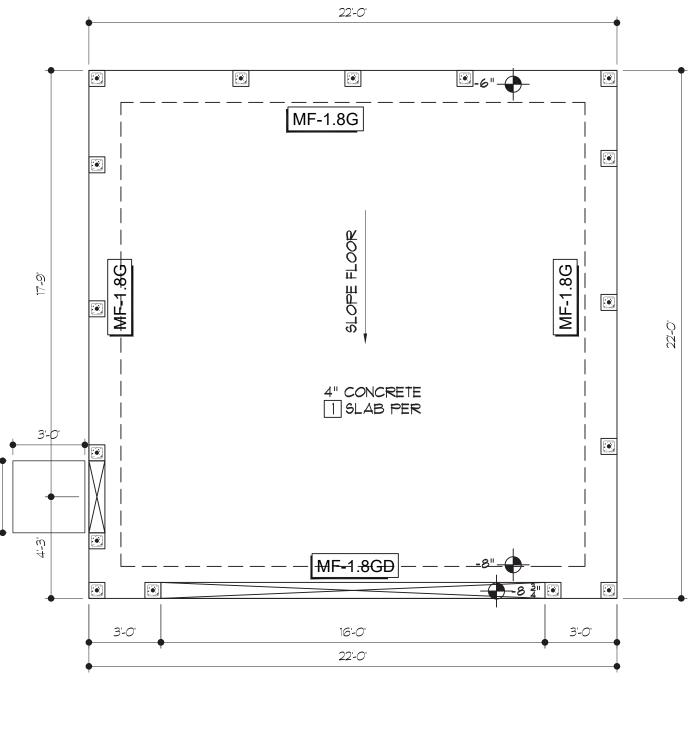




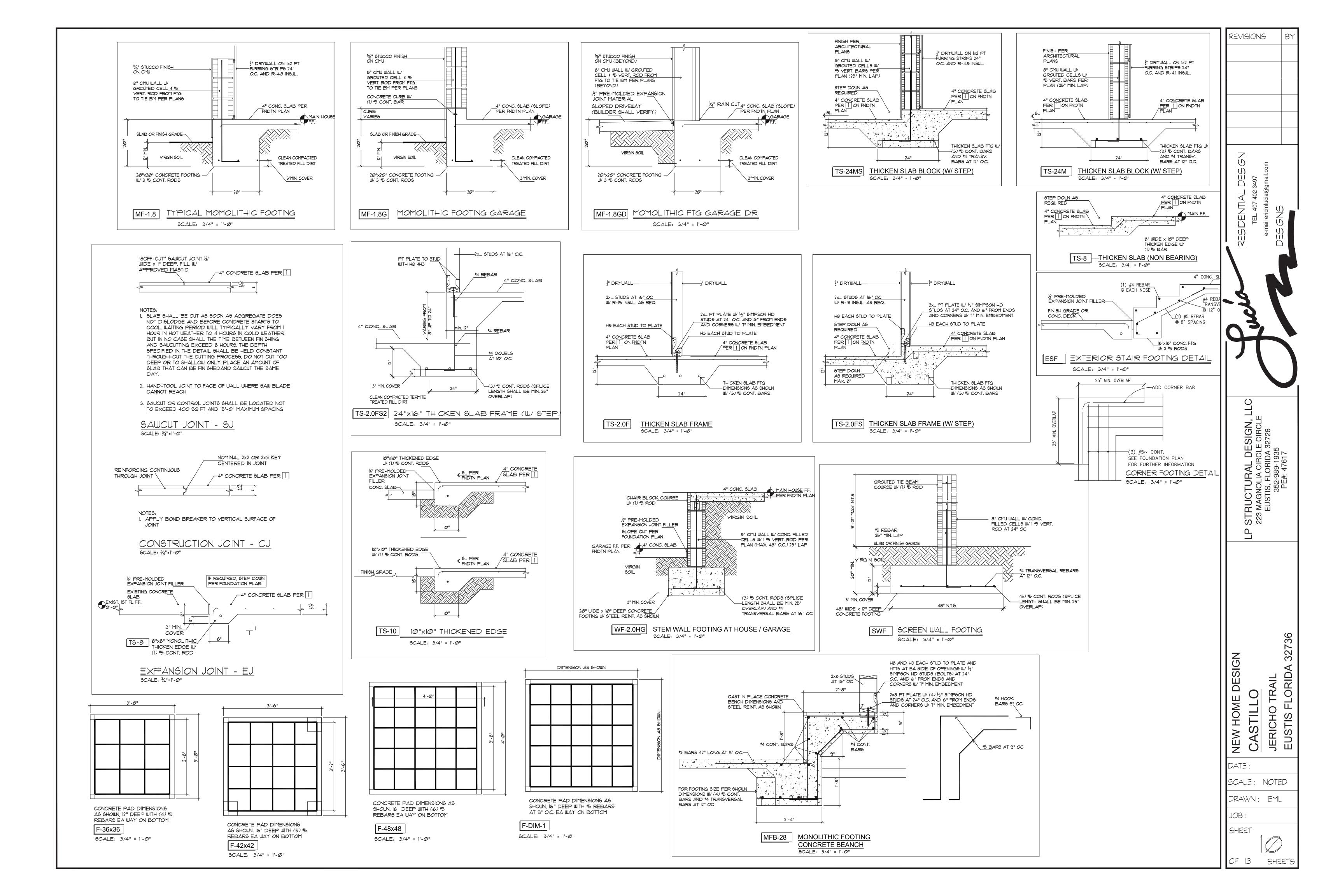
1. UNLESS OTHERWISE SPECIFICALLY STATED HEREIN, THE ELECTRICAL PLAN(S) ARE ONLY FOR GENERAL DESIGN INTENT AND HAVE BEEN COMPILED TO MEET PERMIT REQUIREMENTS OF THE AUTHORITIES HAVING JURISDICTION. ACTUAL QUANTITY, TYPE, AND PLACEMENT OF OUTLETS, SWITCHES, FIXTURES, AND ALL OTHER RELATED ELECTRICAL EQUIPMENT SHALL BE DETERMINED BY THE CONTRACTOR AND OWNER. INSTALLATION SHALL BE IN ACCORDANCE WITH ALL APPLICABLE CODES.	
2. CONTRACTOR SHALL VERIFY WITH POWER COMPANY THE LOCATION + OF SERVICE AND SHALL LOCATE METER AND PANEL AS REQUIRED.	
3. ALL WIRES SHALL BE THW COPPER, UNLESS NOTED OTHERWISE.	
4. WHERE REQUIRED BY OTHER CODES, SERVICE AND FEEDER CONDUCTORS SHALL BE COPPER OF EQUAL AMPACITY.	
5. ALL BRANCH CIRCUITS IN RACEWAY OR NON-METALLIC SHEATHED CABLE.	
6. COORDINATE RACEWAY INSTALLATIONS WITH OTHER TRADES PRIOR TO CONSTRUCTION.	
<ol> <li>VERIFY ALL CONDUCTORS AND BREAKERS WITH EQUIPMENT MANUFACTURERS SPECIFICATIONS.</li> </ol>	
8. PROVIDE DISCONNECT SWITCH SIZE AS REQUIRED BY LOAD AND UNITS.	
9. PROVIDE NON-FUSIBLE GENERAL DUTY SAFETY SWITCHES AT A/C EQUIPMENT, AND AT PUMPS NOT VISIBLE FROM CIRCUIT BREAKER PANEL AND AS PER MANUFACTURER'S RECOMMENDATIONS.	
10. PROVIDE GROUND FAULT INTERRUPT (GFI) BREAKERS FOR ALL BATHROOM, GARAGE AND EXTERIOR OUTLETS AS SHOWN.	
11. ELECTRICAL FIXTURES, TRIM AND APPLIANCES SHALL BE 'UL' APPROVED AND SELECTED BY OWNER.	
12. PROVIDE PRE-WIRED TELEPHONE AND TELEVISION (CABLE TV) OUTLETS.	$\begin{bmatrix} \Psi & \Psi \\ \Psi & \Psi \end{bmatrix}$
13. WIRE KITCHEN AND FAMILY ROOM SEPARATELY.	
14. ELECTRICAL SERVICE SIZE SHALL BE DESIGNED BY THE ELECTRICAL CONTRACTOR. THIS PLAN SHALL BE USED AS A GUIDE, POWER REQUIREMENTS SHALL BE DETERMINED BY TOTAL LOAD OF THE HOUSE.	
15. PROVIDE AFCIS (ARC FAULT INTERRUPTERS) IN ALL DWELLING UNIT BEDROOMS PER NEC.	
BEDROOMS FER NEC. 16. INSTALL SMOKE DETECTORS IN EACH SLEEPING ROOM. INSTALL COMBO. SMOKE 4 CO2 DETECTORS AT TOP AND BOTTOM OF STAIRS AND WITHIN 10'-0' OF SLEEPING ROOMS. ALL DETECTORS ARE TO BE INTERCONNECTED AND HAVE BATTERY BACKUPS.	











	8" P	NLOA5		RESIR	ESSED	U-LII	NIELS	
CAST-CAETE				RAV				
LENGTH	8U8				8F2Ø-ØE		-	· · ·
		8F8-1B	8F12-18		8F2Ø-1B			8F32-1B
2'-10"(34") PRECAST	23Ø2	3166	4473	6039	7526	9004	10472	11936
		3166	4473	6039	7526	9004	10472	11936
3'-6" (42") PRECAST	23Ø2	3138	3377	4689	6001	7315	8630	9947
		3166	4473	6039	7526	9004	10472	11936
4'-0"(48") PRECAST	2029	2325	2496	3467	4438	5410	6384	7358
		2646	4473	6039	7526	9004	10472	11936
4'-6" (54") PRECAST	1651	דפרו	1913	2657	34Ø3	4149	4896	5644
		2170	4027	6039	7526	9004	10472	9668
5'-4" (64") PRECAST	1184	1223	1301	1809	2317	2826	3336	3846
	10-T	1665	2889	5057	6096	5400	6424	7450
5'-10"(10") PRECAST	972	1000	1059	1474	1889	23Ø4	2721	3137
	J 12	1459	2464	4144	5458	4437	5280	6122
6'-6"(18") PRECAST	937	1255	2101	3263	2746	3358	3971	4585
	100	1255	21Ø1	3396	5260	7134	8995	6890
1'-6" (90") PRECAST	767	1029	1675	2385	1994	2439	2886	3333
I - C ( SC / RECAST	101	1029	1675	2610	3839	5596	6613	5047
9'-4" (112") PRECAST	573	632	1049	1469	121Ø	1482	1754	2Ø27
5-4" (112") PRECASI		768	1212	1818	2544	3469	4030	3127
		482	802	1125	915	1122	1328	1535
10'-6'(126") PRECAST	456	658	1025	1514	2081	2774	3130	24Ø4
		598	935	1365	1854	2355	1793	2075
11'-4" (136") PRECAST	445	598	935	1365	1854	2441	3155	4044
		545	864	1254	1689	2074	1570	1818
12'-Ø"(144") PRECAST	414	555	864	1254	1693	2211	2832	3590
		427	726	1028	1331	1635	1224	1418
13'-4"(160")PRECAST	362	485	748	1076	1438	1855	2343	2920
		381	648	919	1190	1462	1087	1260
14'-0"(168") PRECAST	338	455	700	1003	1335	1714	2153	2666
		NR	NR	NR	NR	NR	NR	NR
4'-8" (176") PRESTRESSEE	? N.R.	465	765	1370	2045	2610	3185	3765
		NR	NR	NR	NR	NR	NR	NR
5'-4" (184") PRESTRESSE	) N.R.	420	695	1250	1855	2370	2890	3410
		NR	NR	NR	NR	NR	NR	NR
1'-4" (208")PRESTRESSE	) N.R.	310	530	950	1400	1800	2200	2600
		NR	NR	NR	NR	NR	NR	NR
3'-4" (232") PRESTRESSE	) N.R.	240	400	750	1090	1400	1720	2030
		NR	NR	NR	NR	NR	NR	NR
21'-4" (256")PRESTRESSE	) N.R.	183	330	610	940	1340	1780	2110
		NR	NR	NR	NR	NR	NR	NR
22'-@"(264")PRESTRESSE	) N.R.							
		160 NR	300 NR	570 NR	87Ø NR	1250 NR	1660 NR	1970 NR
1								

8" PRECAST & PRESTRESSED U-LINTELS	CAST-CRETE®
UFLIFI         LATERAL           TYPE         8F8-IT         8F12-IT         8F12-IT         8F20-IT         8F22-IT         8F32-IT         8F32-IT	
LENGTH 8F8-2T 8F12-2T 8F16-2T 8F20-2T 8F20-2T 8F24-2T 8F32-2T	
2'-10"(34") PRECAST 2121 2184 3981 5190 6401 1630 8851 2021 2021 2121 2184 3981 5190 6401 1630 8851 2021 2021	FE LOAD TABLES
	L LVAD IADLLJ
4-0"(48") PRECAST 1878 1925 2750 3583 4422 5264 6110 938 938	
4-6 (54) PRECAST 1660 1705 2435 3171 3913 4658 5406 121 121 1393: 1484 2110 2741 3375 4010 4648	R GRAVITY, UPLIFT & LATERAL LOADS
5'-4" (64") PRECAST 1393 1437 2050 2670 3293 3920 4549 505 505	AST W/ 2" RECESS DOOR U-LINTELS 8" PRECAST W/ 2" RECESS DOOR U-LINTELS
5'-10"(10") PRECAST 1212 1315 1875 2441 3010 3583 4157 418 418 1011 1000 11733 235 0 3760 3390 2010	GRAVITY LATERAL
1141 1182 1684 2192 2703 3216 3732 10 00 00 00 00 00 00 00 00 00 00 00 00	
1'-6" (90") PRECAST 990 1029 1466 1907 2351 2797 3245 591 657	6-IB 8RF10-IB 8RF14-IB 8RF18-IB 8RF22-IB 8RF26-IB 8RF30-IB
$9'-4''(112'') PRECAST  \frac{80!^{*}}{612}  \frac{612}{980}  \frac{1269}{1550}  \frac{1560}{1910}  \frac{1852}{2211}  \frac{2144}{2634}  454  630  \frac{1489}{4'-4''}  \frac{1489}{(52'')}  \frac{1489}{188}  \frac{1889}{188}  \frac$	3053       2982       3954       4929       5904       6880         3071       3412       4982       6472       7947       9416       10878
$\frac{10'-6'(126'') \ PRECAST}{116} \ \frac{118''}{116} \ \frac{438'}{1039} \ \frac{1389}{1389} \ \frac{1111}{2034} \ \frac{2358}{2358} \ \frac{396}{396} \ \frac{493}{493}$	49 2782 2714 3600 4487 5375 6264 4'-6" (54") PRECAST 1192 1507 2311 3121 3937 4756 5577 853 853
11-4" (136") PRECAST 666 535 905 1295 1595 1896 2198 363 556	32 1602 1550 2058 2566 3075 3585 5'-8" (68") PRECAST 924* 1172 1795 2423 3055 3689 4325 501 501
12'-Ø"(144") PRECAST 631 486 818 1209 1514 1799 2086 340 494	53 2162 4074 6472 6516 5814 6839 Te 1500 144e 1924 2400 2876 3352
13'-4"(160") PRECAST  573 409 682 1004 1367 1637 1897 302 398 5'-10"(10") PRECAST 735 118 5'-10"(10") PRECAST 735 118 118	03 2051 3811 6472 6516 5450 6411 5-10°(10°) FRECAST 896 1099 1690 2288 2891 3497 4106 469 469
14'-0"(168") PRECAST 548 378 629 922 1254 1567 1816 286 360 6'-8" (80") PRECAST 822 9	01 1617 2933 4100 6730 8177 6707 6707 6707 80"/PRECAST 778 956 1468 1987 2509 3035 3563 830 1100
14'-8" (176") PRESTRESSED 243 352 582 852 1156 1491 1742 N.R. 357	61       1371       2252       1958       2451       2944       3439         64       1371       2329       3609       5492       6624       5132         7'-6"       (90")PRECAST       688       697       1325       1810       2280       2753       3227       710       941
15'-4"(184") PRESTRESSED 228 329 542 T91 10T2 1381 16T6 N.R. 32T	20 834 1253 1071 1342 1614 1886 9'-8" (116") PRECAST 533* 433 808 1123 1413 1704 1995 516 614
17'-4" (208")PRESTRESSED 188 276 449 649 874 1121 1389 N.R. 255	35 928 1497 2179 2618 3595 2875 *REDUCE VALUE BY 15% FOR GRADE 40 FIELD REBAR
19'-4" (232") PRESTRESSED 165 207 313 401 490 578 667 165 239 383 550 736 940 1160 N.R. 204	
21'-4" (256") PRESTRESSED 142 212 336 411 635 801 993 N.R. 112 SPECIFIED COMPOSITE LINTEL DEPTH IS 140 190 268 343 419 493 569 THE MINIMUM ACCEPTABLE. ANY EXTRA	
22'-0"(264")PRESTRESSED 140 160 263 343 410 433 360 N.R. 161 COURSES OF BLOCK ABOVE LINTEL ARE	
24-0-(288") PRESTRESSED 124 186 290 408 538 680 833 N.R. 135 ABOVE P.C. LINTEL ARE FILLED W/ GROUT.	
•REDUCE VALUE BY 25% FOR GRADE 40 FIELD REBAR	
GINFERING BODE LICE	
mortar head and bed joints. 8F16-1B 8RF6-0B/1T 8F8-1B/1T 8F8-0B/1T 8RF14-1B/ lintels as required.	
of lintel must comply with the architectural and/or structural drawings. *5 REE	8RF30-1B/1T 8F32-1B/1T
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of line lines computed with 5-1/2 'long notches at the ends to accommodate reinforcing and grouting. If one case of the source commodate reinforcing and grouting. If one case of the source commodate reinforcing and grouting. If added resolution of the lines of concrete masorry units. It needs to be located in the provided in composite lines in line of concrete masorry units. It needs at the source commodate line in the original design analysis per ACI 318 and ACI 530 The source commodate lines in line of concrete masorry units. It needs at the source commodate lines in line of concrete masorry units. It needs at the source commodate lines in the original design analysis per ACI 318 and ACI 530 The source commodate lines in lines of concrete masorry units. It needs at the source commodate lines in lines of concrete masorry units. It needs at the source commodate lines in lines of concrete masorry units. It needs at the source commodate lines in lines of concrete masorry units. It needs at the source commodate lines of the lines of concrete masorry units. It needs at the source commodate lines of the lines	<section-header><section-header><section-header></section-header></section-header></section-header>

## <u>PRE-CAST LINTEL ENG</u> PER CAST-CRETE

## MATERIALS

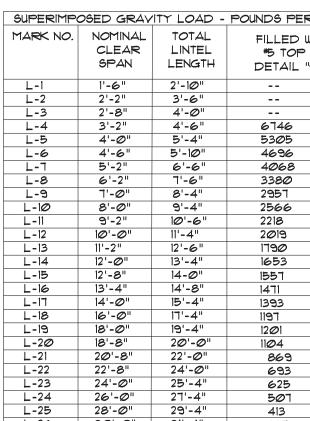
- 1. f'c precast líntels = 3500 psi.
- 2. f'c prestressed lintels = 6000 psi.
- f'c grout = 3000 psi w/ maximum 3/8" aggregate.
   Concrete masonry units (CMU) per ASTM C90 w/ minimum net area compressive strength = 1900 psi.
- 5. Rebar provided in precast lintel per ASTM A615 GR60. Field rebar per ASTM A615 GR40 or GR60. 6. Prestressing strand per ASTM A416 grade
  270 low relaxation.
  7. 7/32 wire per ASTM A510.
  8. Mortar per ASTM C270 type M or S.

SAFE LOAD TABLE NOTES

## GENERAL

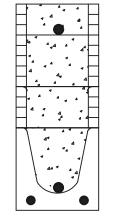
- 1. Provide full mor 2. Shore filled linte
- 3. Installation of li
- 4. Lintels are manuf vertical cell rei
- 5. All líntels meet
- longer with a no
- 6. Bottom field ad 7. 7/32" diameter w 8. Cast-in-place co
- 9. Safe load rating
- All values based on minimum 4" bearing. Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6-1/2". Safe loads for all recessed lintels based on 8" nominal bearing.
- 2. N.R. = Not Rated.
- 3. Safe loads are total superimposed allowable load on the section specified.
- 4. Safe loads based on grade 40 or grade 60 field rebar. 5. Additional lateral load capacity can be obtained by the designer by providing
- addional reinforced masonry above the precast lintel.

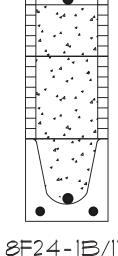
- 6. One #7 rebar may be substituted for two #5 rebars in 8" lintels only.
  7. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from the face of sup 8. For composite lintel heights not shown, use safe load from next lower height.
  9. All safe loads in units of pounds per linear foot.

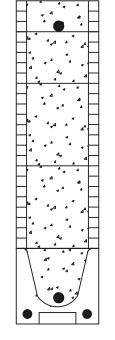


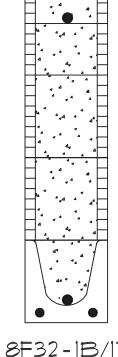
TYPICAL POWER BOX LINTEL SECTION







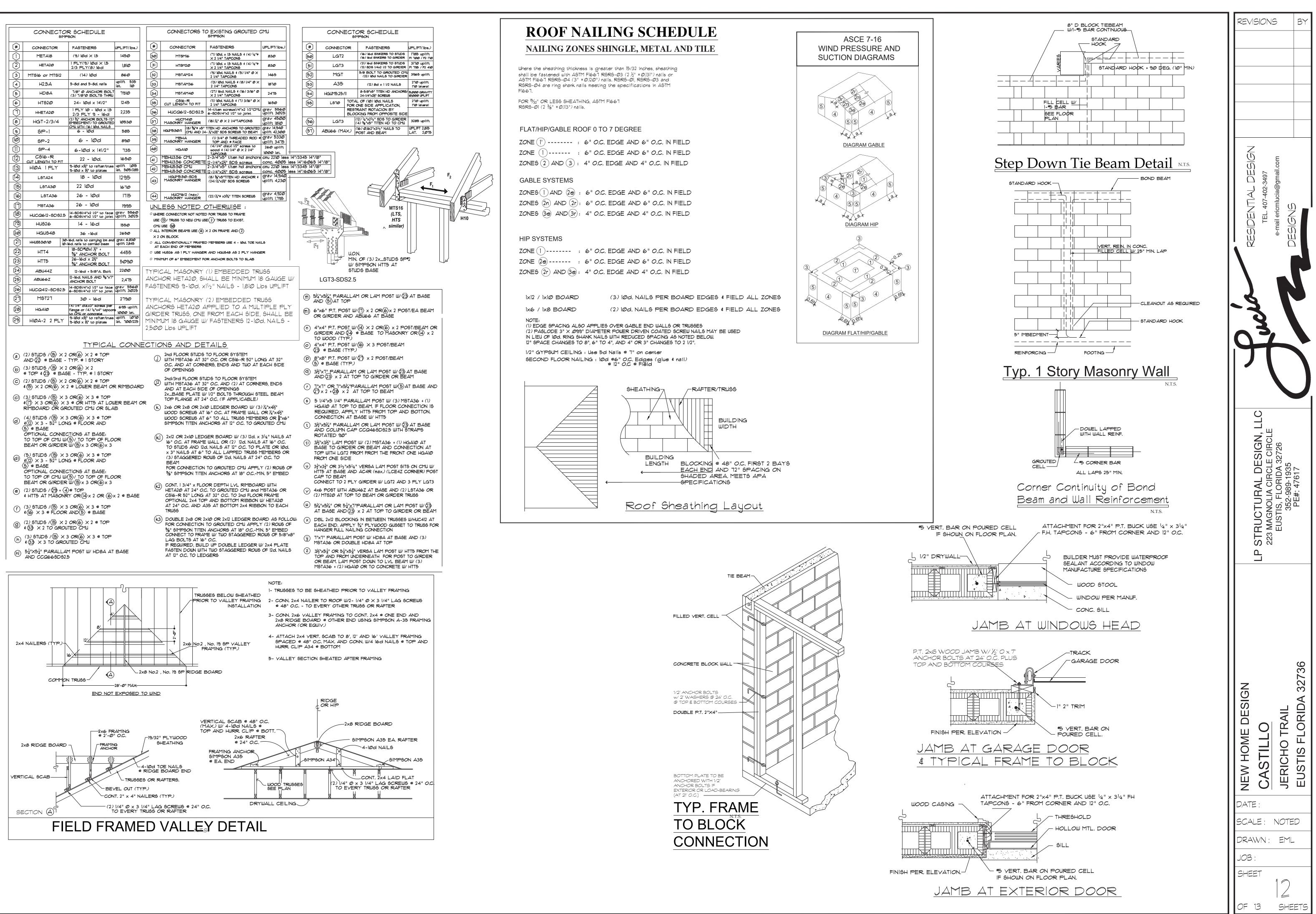


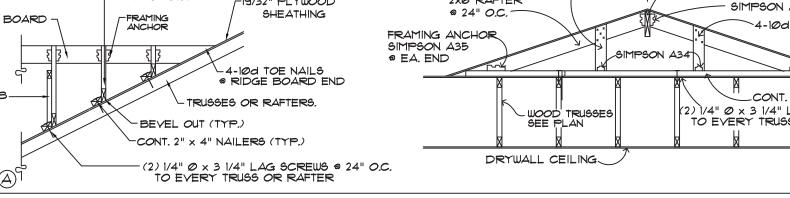




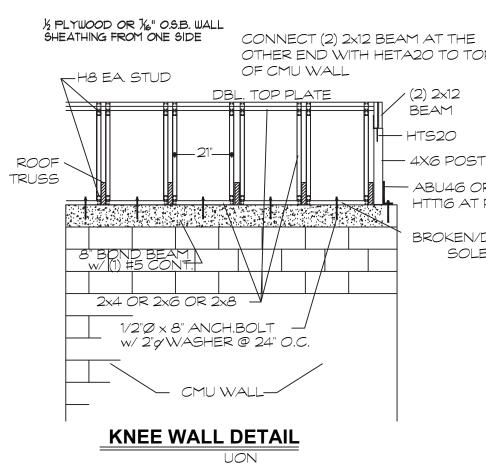
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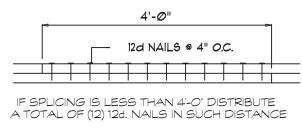
SHEETS



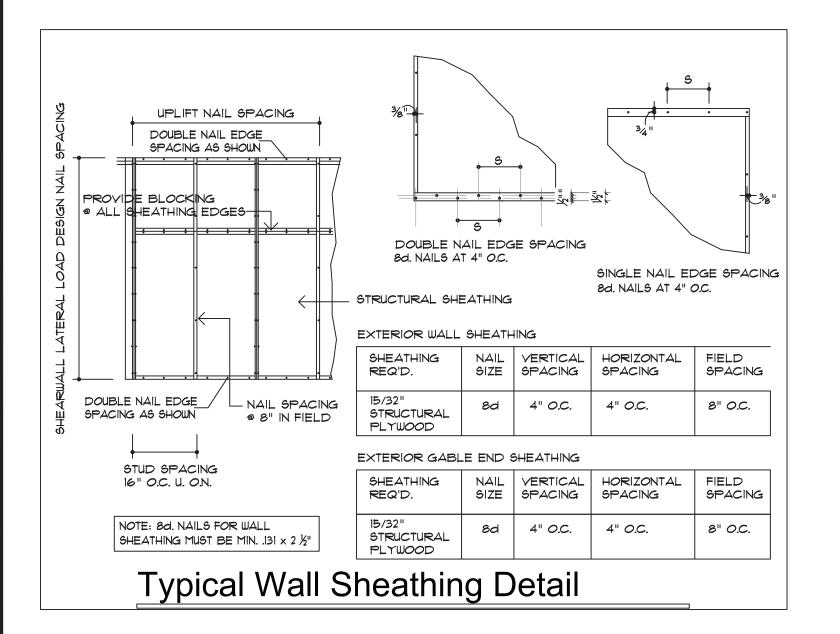


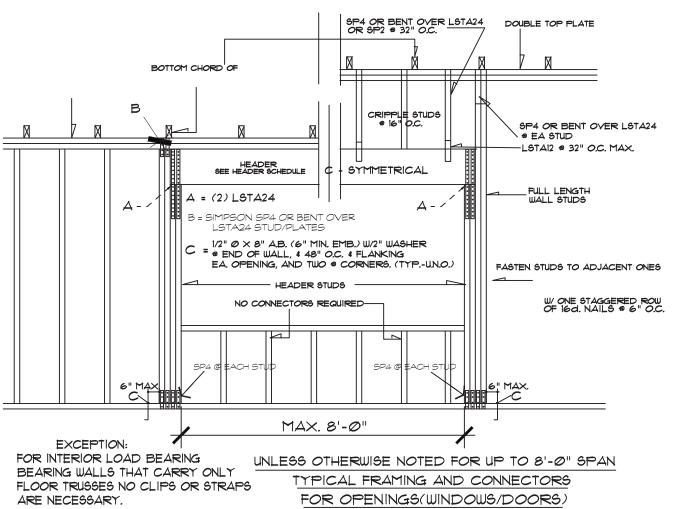
LIMITING HEIGHTS OF 2	2" STUDS					
FOR HIGH FRAMING CONDITIONS, THE APPLICATION OR CONNECTION OF AN INTERMEDIATE FRAMING SUCH AS FLOOR JOISTS, CEILING JOISTS, ETC., ALLOW TO REDUCE THE ALLOWABLE HEIGHT FOR STUDS						
SIZE STUD MATERIAL AT O.C. SPACING	MAXIMUM HEIGHT					
2"x4" SPRUCE, FIR 24" O.C.	8'-0"					
2"x4" SPRUCE, FIR 16" O.C.	9'-0"					
2"x4" SPRUCE, FIR 12" O.C.	10'-0"					
2"x4" SOUTHERN PINE, FIR 24" O.C.	9'-0"					
2"x4" SOUTHERN PINE, FIR 16" O.C.	10'-9"					
2"x4" SOUTHERN PINE, FIR 12" O.C.	12'-4"					
2"x6" SPRUCE, FIR 24" O.C.	11'-4"					
2"x6" SPRUCE, FIR 16" O.C.	13'-9"					
2"x6" SPRUCE, FIR 12" O.C.	16'-0"					
2"x6" SOUTHERN PINE, FIR 24" O.C.	13'-9"					
2"x6" SOUTHERN PINE, FIR 16" O.C.	17'-0"					
2"X6" SOUTHERN PINE, FIR 12" O.C.	19'-4"					
2"x8" SPRUCE, FIR 24" O.C.	14'-9"					
2"x8" SPRUCE, FIR 16" O.C.	18'-0"					
2"x8" SPRUCE, FIR 12" O.C.	21'-0"					
2"x8" SOUTHERN PINE, FIR 24" O.C.	18'-0"					
2"x8" SOUTHERN PINE, FIR 16" O.C.	22'-3"					
2"x8" SOUTHERN PINE, FIR 12" O.C.	25'-8"					



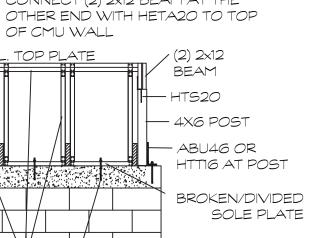




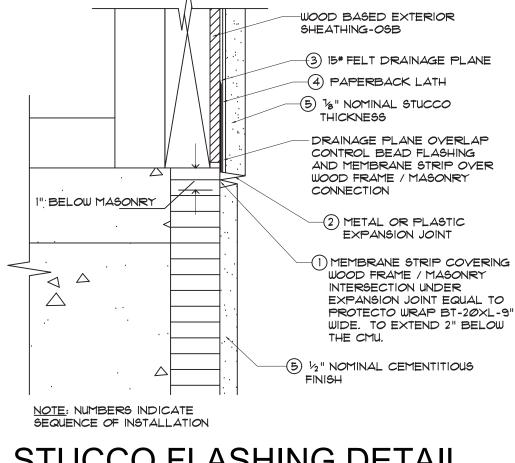




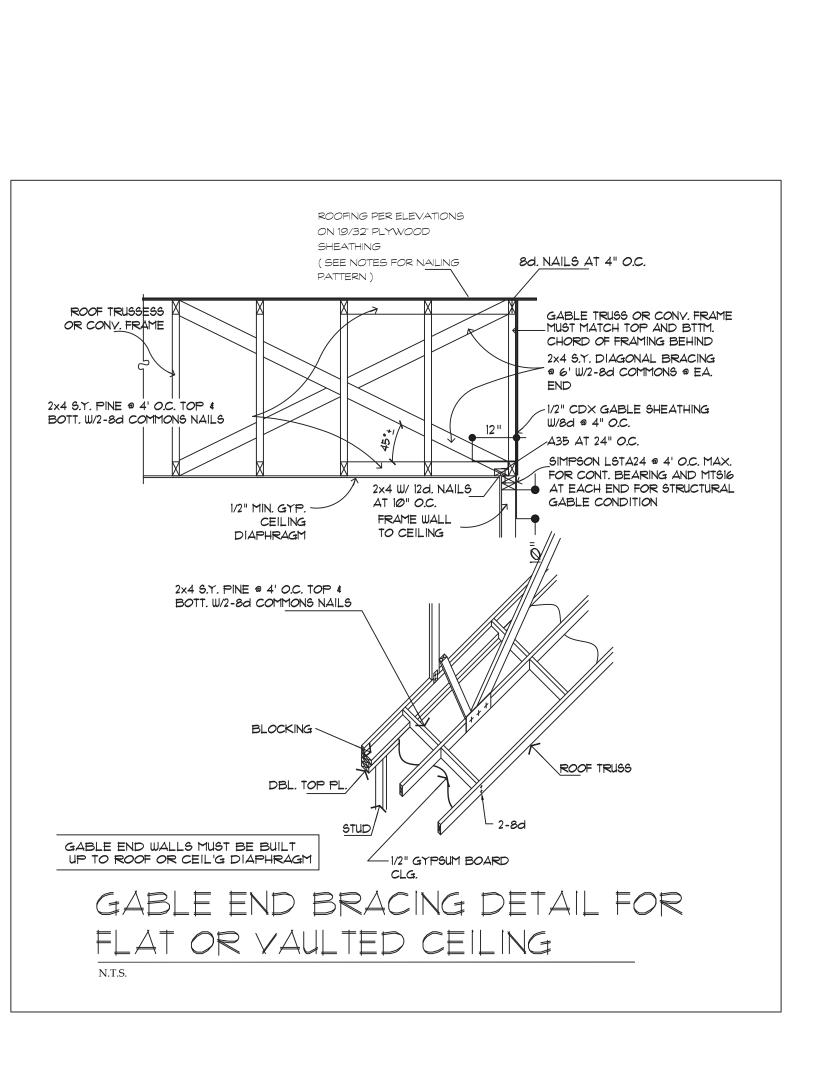
FOR EXTERIOR OR INTERIOR LOAD BEARING WALL



2-2x STUDS UNDER LINTEL OPENINGS LE THAN 5'-0" HEADER SC	TOP PL	 6d SPIKES @ 12" OP PLATE •CONT. AILED ' PLYWD.	<ol> <li>USE HEADER SIZES OTHERWISE NOTED</li> <li>PRIMARY FRAMING WERE SIZED USING 1800 'FB' EXTREMI 90 'FV' HORIZONTA 16E 'E' MODULES O</li> <li>JOIST, RAFTERS, LI USING: 1200 'FB' EXTREME 90 'FV' HORIZINTA 16E 'E' MODULES O</li> </ol>	ON FRAMING PL (BEAMSGIRDER E FIBER IN BEND AL SHEAR OF ELASTICITY NTELS, ETC. WERE E FIBER IN BEND - SHEAR	.AN RS,ETC ING(SI E SIZE	NGLE D					
OPENING WIDTH 0'-0" TO 3'-0" 3'-1" TO 5'-0"	OR SHEARWALL 2-2x8'S + PLYWD. FLITCH 2-2x10'S +	NON-BEARING WALLS 2-2x4'S 2-2x4'S	MINIMUM WALL AN UPLIFT CONNECTION F					-		<b>T</b> )	
5'-1" TO 7'-0"	PLYWD. FLITCH 2-2x12'5 +	2-2×6'5	AT POINTS 'A'(TOP ANI HEADER STUDS, UPLI		3	6	9	12	15	18	
	PLYWD. FLITCH		IS REQUIRED AT EACH	END OF HEADER	_	NUMBER OF HEADER STUDS * SUPPORTING END OF HEADER					
7'-1" TO 9'-0"	2-2x12 W/ 1/2" PLYWD. FLITCH	2-2x8'5	AND AT BOTTOM OF HEADER STUDS IN ADDITION TO CONNECTORS AT WALL STUDS		1	2	2	2	2	2	
		CONT. TO	UNSUPPORTED WALL HEIGHT	STUD Spacing					ENGTH S		
•		4TE 	10' OR LESS	12 " 16 " 24 "	221	222	332	332	N M M	332	
	۲ \	6d SPIKES @ 12" "OP PLATE	GREATER THAN 10'	12" 16" 24"	221	222	33 2	4 3 2	<u>т</u> 4 и	ち 4 ろ	
2-2x STUDS UNDER LINTEL OPENINGS LE THAN 5'-0"		AILED		R STUD SHALL NOT BE PORTED BY A SUITABL					]		
HEADER SC	CHEDULE FOR	R 6" WALL									
OPENING WIDTH	BEARING WALL OR SHEARWALL	NON-BEARING WALLS									
0'-0" TO 3'-0"	3-2x10'S + PLYWD. FLITCH	3-2x4'S									
3'-1" TO 5'-0"	3-2x12'S + PLYWD. FLITCH	3-2×6'5									
5'-1" TO 7'-0"	3-14" LVL	3-2x8'5									
7'-1" TO 9'-0"	3-14" LVL	3-2x10'5									



## STUCCO FLASHING DETAIL @ CMU / FRAME INTERFACE





ARE NECESSARY. ONLY ANCHORAGE TO SLAB IS REQUIRED ACCORDING TO "C"