RELEASE DATE:

JULY 15, 2021 - TO TRUSSES SEPTEMBER 22, 2021 - TO ENGINEER **OCTOBER 9TH - ENGINEERING COMPLETED** FEBRUARY 24, 2022 - REV.TO ENGINEER JULY 07, 2022 - COMMENTS - SHEETS 2&3

A CUSTOM DESIGN ALLEN RESIDENCE

COUNTY OF LAKE STATE OF FLORIDA

ALL FEDERAL, STATE & LOCAL CODES, ORDINANCES, AND REGULATIONS, ETC. SHALL BE CONSIDERED AS PART OF THE SPECIFICATIONS OF THIS BUILDING: AND ARE TO BE ADHERED TO EVEN IF THEY ARE IN VARIANCE WITH THE PLAN.

DESIGNER AND ENGINEER ASSUME NO RESPONSIBILITY OVER ANY PHASE OF CONSTRUCTION OR COMPLETED BUILDING.

WIND PRESSURE AND

SUCTION DIAGRAMS

TERMITE SPECIFICATIONS

SECTION R318 PROTECTION AGAINST TERMITES

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED CONSTRUCTION (SEE SECTION 202, REGISTERED TERMITICIDE), UPON COMPLETION OF THE ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

1. METHOD OF TREATMENT SHALL BE APPROVED BY THE GOVERNING JURISDICTION "LIQUID PRODUCT APPROVAL DATA MUST BE ON FILE WITH THE BUILDING DEPARTMENT. BORATE OR BOR-A-COR" PRODUCT METHODS MUST BE DETERMINED AT PERMIT STAGE AND 2. PRESSURE TREATED LUMBER THAT HAS BEEN CUT OR DRILLED THAT EXPOSES UNTREATED PORTIONS OF WOOD ARE REQUIRED TO BE FIELD TREATED TO PREVENT INSECT INFESTATION. 3. OPTIONAL BORATE APPLIED TO ALL FRAME MEMBERS WITHIN 24" A.F.F.

-- NOTICE TO BUILDER AND ALL SUBCONTRACTORS--

IT IS THE INTENT OF THE ENGINEER LISTED IN THE TITLEBLOCK OF THESE DOCUMENTS THAT THESE DOCUMENTS BE ACCURATE, PROVIDING LICENSED PROFESSIONALS CLEAR INFORMATION. EVERY ATTEMPT HAS BEEN MADE TO PREVENT ERROR. THE BUILDER AND ALL SUBCONTRACTORS ARE 1. REVIEW ALL THE INFORMATION CONTAINED IN THESE DOCUMENTS, PRIOR TO THE

COMMENCEMENT OF ANY WORK. THE ENGINEER ARE NOT RESPONSIBLE FOR ANY PLAN ERRORS, OMISSIONS, OR MISINTERPRETATIONS UNDETECTED AND NOT REPORTED TO THE ENGINEER

2. SHALL STRICTLY OBSERVE ALL APPLICATION CODES DURING THE COURSE OF CONSTRUCTION INCLUDING ALL STATE, CITY, AND COUNTY BUILDING, ZONING, ELECTRICAL, MECHANICAL, PLUMBING AND FIRE CODES. CONTRACTOR SHALL VERIFY ALL CODE REQUIREMENTS PRIOR TO

3. THE ARCHITECT / ENGINEER SHALL NOT BE RESPONSIBLE FOR SAFETY PROCEDURES, THE MEANS AND METHODS OF CONSTRUCTION, TECHNOLOGIES, OR THE CONTRACTION TO CARRY OUT THE WORK IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS OR RELATED CODES. 4. THE FRAMING PLAN SHOWN INDICATES THE "TRUSS SYSTEM" AND IS THE RESPONSIBILITY OF THE TRUSS SYSTEM ENGINEER (DESIGN PROFESSIONAL OF RECORD). THE TRUSS DESIGN ENGINEER (DELEGATED ENGINEER) HAS FINAL, RESPONSIBILITY FOR EACH INDIVIDUAL TRUSS AND TRUSS PROFILE, AND IS TO SUBMIT A FINAL SET OF TRUSS ENGINEERING SIGNED AND SEALED TRUSS DRAWINGS TO DESIGN PROFESSIONAL OF RECORD FOR REVIEW PRIOR TO FABRICATION 5. ANY DISCREPANCY OR ERROR IN DIMENSIONS OR NOTES WITH IN THIS PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGN PROFESSIONAL FOR CLARIFICATION PRIOR TO

CONSTRUCTION. 6. ALL CONSTRUCTION MUST BE IN ACCORDANCE TO THE INFORMATION FOUND IN THESE DOCUMENTS. ANY QUESTIONS REGARDING THE INFORMATION FOUND IN THESE PLANS SHOULD BE DIRECTED TO OUR QUALITY ASSURANCE MANAGER AT 321-972-0491 IMMEDIATELY. NO BACK CHARGES WILL BE CONSIDERED FOR REIMBURSEMENT BY THE THE ENGINEER WITHOUT ADVANCED NOTIFICATION AND APPROVAL BY THE ENGINEER. PAYMENTS WILL BE MADE IN

PER FBC 2020 7TH EDITION, RESIDENTIAL VOLUME R703.7.1: LATH AND LATH ATTACHMENTS SHALL BE OF CORROSION-RESISTANT MATERIALS. EXPANDED METAL OR WOVEN WIRE LATH SHALL BE ATTACHED WITH I\"LONG (38MM), I I GAUGE NAILS HAVING A 7/6" (II.IMM) HEAD OR IN LONG (22.2MM), IG GAUGE STAPLES, SPACED IN ACCORDANCE WITH ASTM C1063 OR C1787, OR AS OTHERWISE APPROVED

CONCRETE CURING AND TESTING

ACCORDANCE TO THE TERMS OF THE AGREEMENT.

CURING: PROTECT CONCRETE FOR 7 DAYS AGAINST MOISTURE LOSS, RAPID TEMPERATURE CHANGE, MECHANICAL INJURY AND INJURY FROM RAIN OR FLOWING WATER. MAINTAIN CONCRETE IN MOIST CONDITION AT TEMPERATURE ABOVE 50 DEGREES F, THROUGHOUT SPECIFIED CURING PERIOD. PROTECT FROM RAPID TEMPERATURE CHANGE AND RAPID DRYING FOR FIRST 24 HOURS FOLLOWING REMOVAL OF TEMPERATURE PROTECTION. START CURING ACTIVITIES AS SOON AS FREE WATER HAS DISAPPEARED FROM SURFACES OF CONCRETE AFTER PLACING AND FINISHING.

TESTING: CONCRETE TESTING FOR THIS PROJECT SHALL BE PAID FOR BY OWNER, AND SHALL CONSIST OF COMPRESSIVE TESTS MADE BY THE LABORATORY IN ACCORDANCE WITH ASTM C-31, FOLLOW ASTM C31 AND MAKE A SET OF SIX (6) STANDARD CYLINDERS FOR EACH 100 CU. YDS. OR FOR EACH DAYS POUR EXCEEDING 5 CU. YDS. TEST PER ASTM C39 AS FOLLOWS: TWO (2) SPECIMENS TESTED AT SEVEN (7) DAYS, ONE (1) AT 14 DAYS. TWO (2) TESTED AT 28 DAYS, AND ONE (1) HELD IN RESERVE. SLUMP TEST SHALL BE MADE IN ACCORDANCE WITH ASTM C-143 FOR EACH DAYS POUR, FOR EACH LOAD, OR AS DIRECTED BY ARCHITECT/ ENGINEER.

REINFORCING STEEL

REINFORCING STEEL SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615-GRADE 60, EXCEPT THAT NEW BILLET STEEL CONFORMING TO ASTM A615-GRADED 40 MAY BE USED FOR COLUMN TIES AND BEAMS STIRRUPS. ALL DETAILING AND ACCESSORIES SHALL CONFORM TO TYPICAL DETAILS SHOWN IN THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES ACI 315, LATEST EDITION".

ALL CONTINUOUS VERTICAL OR HORIZONTAL BARS IN FOOTINGS, FOUNDATIONS WALLS, SLABS AND OTHER CONCRETE SHALL BE LAP-SPLICED, WHERE NECESSARY OR DESIRABLE, BY WIRING TOGETHER IN CONTACT, LENGTH OF ALL #5 LAPS SHALL BE 40-BAR DIAMETERS OR 2'-1" MINIMUM, WHICHEVER IS GREATER (EXCEPT AS NOTED BY DRAWINGS). ALL BARS AT END OF CONTINUOUS FOOTINGS OR BEAMS SHALL BE CONT. TO FAR SIDES OF INTERSECTING ELEMENTS.

ALL SLABS ON GRADE SHALL BE 4" THICK AND REINFORCED WITH 6 X 6 - WI.4 X WI.4 W.W.F. UNLESS OTHERWISE NOTED. LAP FABRIC 8" AT EDGES AND ENDS AND PROVIDE ADDITIONAL REINFORCING WHERE SHOWN ON DRAWINGS. PLACE MESH IN CENTER OF SLAB. MOISTURE BARRIER BENEATH FLOOR SLABS SHALL BE 6 MIL POLYETHYLENE. USE FLAT SHEETS OF WELDED WIRE FABRIC. ROLLS WILL NOT BE PERMITTED

FOUNDATIONS

GEOTECHNICAL ENGINEERING EVALUATION AND SUBSURFACE EXPLORATION SHALL PERFORM BY OWNER'S GEOTECHNICAL CONSULTANT.

MAXIMUM ALLOWABLE SOIL PRESSURE IS ASSUMED TO BE 2000 POUNDS PER SQUARE

SPREAD FOOTINGS SHALL BEAR ON SOIL COMPACTED TO A DENSITY OF AT LEAST 95 % OF MODIFIED PROCTOR MAXIMUM DENSITY (A.S.T.M. D1557), FOR ALL REQUIRED FILL AND FOR AT LEAST 1'- 0" BELOW FINISHED FLOOR UNLESS MORE STRINGENT REQUIREMENTS ARE RECOMMENDED BY OWNERS GEOTECHNICAL CONSULTANT.

GENERAL STRUCTURAL NOTES

CAST IN PLACE REINFORCED CONCRETE

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI (SLABS) 3000 PSI (COLUMNS AND BEAMS), A SLUMP OF 5" PLUS OR MINUS 1". AND HAVE 2 TO 5% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.63 2. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS. 3. HORIZONTAL FOOTING BARS SHALL BE BENT 25" AROUND CORNERS OR CORNER BARS WITH A 25" LAP PROVIDED EA WAY.

5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064A / A1064M. WWF SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 6". POLYPROPYLENE FIBERS FOR SLABS ON GRADE TO BE MIN 1.5 LBS OF FIBER PER CUBIC YARD

6. ALL REINFORCING STEEL / STIRRUPS AND TIES SHALL BE NEW DOMESTIC DEFORMED BARS FREE FROM RUST.SCALE & OIL & SHALL MEET ASTM A615/ A615M GRADE 40 U.N.O. REINFORCING FOR FOOTING SHALL BE SUPPORTED ON PRE-CAST CONCRETE PADS. STEEL WIRE OR PLASTIC SUPPORT. TOP

MUST FIRST CONTACT THE ENGINEER OF RECORD FOR WRITTEN APPROVAL. 8. WHERE PROJECT IS TO BE LOCATED IN KNOWN RADON GAS PREVALENT AREAS, APPENDIX "F" OF THE FLORIDA BUILDING CODE 7TH EDITION (2020) IS TO BE IMPLEMENTED. F303.4 CONCRETE STRENGTH IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I. THEREFORE, ANY AND ALL NOTES ON THESE PLANS THAT INDICATE 2500 P.S.I. SHALL BE REPLACED WITH 3000 P.S.I. FOR THE CONCRETE STRENGTH.

MASONRY

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90-014, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1500 PSI)

4. GRADE 40 U.N.O. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH THE CELLS FILLED WITH COARSE GROUT 5. GRADE 40 U.N.O.VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 DIA OR 10FT

7. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM, PLASTIC SCREEN, METALLATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW OF GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED. 8. TEMPORARY BRACING AND SHORING OF WALL TO PROVIDE STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TYPICAL FILLED CELL REINFORCING SIZE AND SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS 10. DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS AND NO CONCENTRATED LOADS FOR (7) DAYS. PER CODE ACI 318-14 11. CONSOLIDATE POURS EXCEEDING 12" IN HEIGHT BY MECHANICAL VIBRATION, AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL

1. ALL EXTERIOR WOOD STUDS WALLS, BEARING WALLS, SHEAR WALLS, AND MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE END BRACING) SHALL BE EITHER AS SPECIFIED IN PLAN OR IN DETAILS. IF CONFLICTS OCCUR BETWEEN PLAN AND DETAILS. THE STRONGEST MATERIAL SHALL BE USED. AT A MINIMUM, ALL WOOD STRUCTURAL FRAMING MEMBERS SHALL BE SPF #2.

2. ALL LUMBER SPECIFIED ON DRAWINGS ARE INTENDED FOR DRY USE ONLY (MOISTURE CONTENT 19% OR LESS), U.N.O. ALL WATERPROOFING AND FIRE SAFETY SYSTEMS ARE THE RESPONSIBILITY OF THE CONTRACTOR AND ARE TO BE DESIGNED AND DETAILED BY OTHERS 3. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION SHIELDS, ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP., U.N.O. 4. MANY OF THE NEW PRESSURE TREATED WOODS USE CHEMICALS THAT ARE CORROSIVE TO STEEL. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE TYPE OF WOOD TREATMENT AND TO SELECT APPROPRIATE CONNECTORS THAT RESIST CORROSION. FOR EXAMPLE, ACQ-C, ACQ-D, CBA-A OR CA-B REQUIRE HOT-DIPPED GALVANIZED OR STAINLESS STEEL FASTENERS. DOT SODIUM BORATE (SBX) DOES NOT.

5. ALL EXPOSED WOOD OR WOOD IN CONTACT WITH EARTH OR CONCRETE TO BE PRESSURE TREATED. 6. UNTREATED WOOD SHALL NOT BE IN DIRECT CONTACT WITH CONCRETE OR MASONRY. SEAT PLATES SHALL BE PROVIDED AT BEARING LOCATIONS

7. SEE PLAN FOR STUD PACK AND BEAM NAILING PATTERNS 8. ALL ENGINEERED LUMBER TO HAVE THE FOLLOWING MIN VALUES U.N.O.

WATER LOSS AND SETTLEMENT HAS OCCURRED, GROUT SHALL BE FLUSH WITH TOP OF WALL.

PARALLAM COLUMNS: 1.8E Fb = 2400 PSI MICROLAM (LVL) BEAMS: 2.0E Fb= 2600 PSI

GLULAM BEAMS: SP/SP 24F-V5 LAYUP (1.7E FB=2400 PSI) MIN. 9. SEE PLAN NOTE FOR ADDITIONAL ROOF, WALL, SHEAR WALL AND FLOOR SHEATHING REQUIREMENTS ALONG W/ NAILING INFORMATION OTHERWISE: 9.1. ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR OR OSB 9.2. FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24) SHEATHING SHALL FINISH FLUSH TO EXTERIOR WALL FACE. 9.3. WALL SHEATHING: 7 1/6" STRUCTURAL I OSB EXPOSURE 1 OR325" RATED OSB EXPOSURE 1. A MINIMUM'8" SPACE IS RECOMMENDED BETWEEN

PANELS AT EDGE AND END JOINTS TO ALLOW FOR EXPANSION. PER R604.3 SHEATHING SHALL NOT BE USED AS WEATHER RESISTANCE BARRIER

UPLIFT CONNECTORS

1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT OR LATERAL FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE COORDINATE THE TRUSS ENGINEER FOR THE LOCATION OF THESE WALLS.AND STRUCTURAL PLANS FOR MORE INFO.

STRUCTURAL STEEL

1. MATERIAL SPECIFICATIONS: WIDE FLANGE SECTIONS: ASTM A992, GRADE 50, Fy=50 KSI TUBE STEEL (HSS): ASTM A500, GRADE B, Fy = 46 KSI PIPE STEEL: ASTM A53, TYPE E OR S, Fy = 35 KSI ALL OTHER STRUCTURAL & MISC. STEEL: A36 Fy=36 KSI STRUCTURAL CONNECTIONS: ALL STRUCTURAL BOLTS TO BE A325N U.N.O. 2. STRUCTURAL BOLTS SMALLER THAN 5/8" DIA. TO BE A307 THREADED ROD SHALL CONFORM TO A36 OR A307 ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307 SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL REINFORCEMENT SHOP DRAWINGS TO BE PROVIDED TO ENGINEER OF RECORD BEFORE FABRICATION FOR REVIEW AND APPROVAL

3. STRUCTURAL CONNECTIONS: ALL STRUCTURAL BOLTS TO BE A325N U.N.O. ALL A325N BOLTS SHALL BE BROUGHT TO A "SNUG-TIGHT" CONDITION, AS DEFINED IN THE SPECIFICATION. SLIP CRITICAL (SC) BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION STRUCTURAL BOLTS SMALLER THAN 5/8" DIA. TO BE A307 THREADED ROD SHALL CONFORM TO A36 OR A307 ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307 SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL REINFORCEMENT SHOP DRAWINGS TO BE PROVIDED TO ENGINEER OF RECORD BEFORE FABRICATION FOR REVIEW AND APPROVAL. WELDED CONNECTIONS: ELECTRODES - E70XX UNO (LOW HYDROGEN). FILLET WELDS SHALL BE 3/16" UNO

4. SUBMIT SHOP DRAWINGS INDICATING ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING, AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOAD, AND TOLERANCES. 5. STRUCTURAL STEEL SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT FOR AREAS WHICH WILL RECEIVE SPRAY-ON FIRE

6. A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

PRE ENGINEERED WOOD TRUSSES

1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS PER 2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. 3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD.

4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS. 5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE

TRUSS MANUFACTURER IN ACCORDANCE WITH THE FRAMING DESIGN LOADS: 6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION. 7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES . SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS, AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 3 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.

8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

FIELD REPAIR NOTES

1. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED WITH 1/2" DIA. EPOXY ANCHORS WITH 7" EMBEDMENT. SIMPSON "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURER'S RECOMMENDATIONS OR SIMPSON 1/2" TITEN HD BOLTS WITH MINIMUM 7" EMBEDMENT. SEE PLAN FOR EMBEDMENT

2. FOR MISSED VERT. DOWELS, DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY (SIMPSON HIGH STRENGTH EPOXY-TIE ANCHORING ADHESIVE) MIXED PER THE MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO THE MANUFACTURER'S SPECIFICATIONS. THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM

3. FOR MORTAR JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING). 4. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4"x 21/4" TITENS TO MASONRY AND (7)-10d NAILS TO TRUSS FOR UPLIFTS LESS THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1720#). IF CORNER STRAP IS MISSED. CONTRACTOR IS TO INSTALL (2) SIMPSON HGAM10 W/ (4) 1/4" x 1 1/2" SDS SCREWS AND (5) 1/4" x 2 1/4" TITENS ONE EACH SIDE OF TRUSS. 5. NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN 3 IN A ROW WITHOUT APPROVAL FROM EOR. IF GIRDER TRUSS CONNECTIONS ARE MISSED, CONTACT THE EOR FOR SUBSTITUTION.

6. IF MISSED, MSTAM36 OR MSTAM40 STRAP IS MISSED FOR 2ND FLOOR JAMB STUD CONNECTION, CONTRACTOR MAY INSTALL SIMPSON HTT5 W/ (26) 16d x 21/2" NAILS AND 5/8" ANCHOR BOLT SET IN SIMPSON HIGH STRENGTH EPOXY W/ MIN 6" EMBEDMENT AND MIN 3" EDGE DISTANCE. CONTACT EOR IF STRAPS ARE MISSED UNDER GIRDER JAMB STUD LOCATIONS.

STRUCTURAL DESIGN CRITERIA

CODE CRITERIA

FLORIDA BUILDING CODE 7TH EDITION (2020) RESIDENTIAL

· FLORIDA FIRE PREVENTION CODE 7TH EDITION (2020) · FLORIDA BUILDING CODE ACCESSIBILITY 7TH EDITION (2020)

NFPA 70-14. NATIONAL ELECTRICAL CODES. (NEC 2017) & 6TH FBCR CH. 34-43 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE - (ACI 318-14).

SPECIFICATIONS FOR STRUCTURAL CONCRETE - (ACI 301-10). BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES - (ACI 530-13). NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION - 2015 EDITION.

WOOD FRAMED CONSTRUCTION MANUAL 2015 EDITION. APA PLYWOOD DESIGN SPECIFICATION 2012 EDITION.

AMERICAN SOCIETY OF CIVIL ENGINEERS: ASCE/SEI 7-16 · ALUMINUM DESIGN MANUAL - 2015 EDITION

GENERAL ROOF LOADING

	ROOF (PSF)	ROOF (PSF)	ROOF (PSF) I	ROOF (PSF)
TOP CHORD LL TOP CHORD DL	20 10	20 10	20 15	20 25
BOTTOM CHORD LL* BOTTOM CHORD DL	0 10	0 10	0 10	0 10
TOTAL (PSF)	40	40	45	55
BOTTOM CHORD LL (OPT) ATTICS W/ LIMITED STORAGE ATTICS W/ HEAVY STORAGE * ATTICS W/ NO STORAGE (NON-CONCURRENT)	20 50 10			

1	NOTE: LL REDUCTIONS ARE ALLOWED PER CODE BUT ONLY WITH WRITTEN
	APPROVAL FROM EOR OR INDICATED ON PLAN

GENERAL FLOOR LOADING 40 (PSF) COMMENTS:

CDECIA	I FLOO	
BOTTOM CHORD DL	5 (PSF)	
BOTTOM CHORD LL	0 (PSF)	
TOP CHORD DL	10 (PSF)	

SPECIAL FLOOR LOADING									
60 (PSF)	COMMENTS:								
40(PSF)	d. A SINGLE CONCENTRATED LOAD								
100(PSF)	APPLIED IN ANY DIRECTION AT ANY								
125(PSF)	POINT ALONG THE TOP.								
200(LBS)(d)	f. BALUSTERS AND PANELS FILLERS								
	60 (PSF) 40(PSF) 100(PSF) 125(PSF)								

SLEEPING ROOMS LIBRARIES - STACK ROOMS	30 (PSF) 150(PSF)		F 50 POUNDS ON AN AREA FO 1 SQ. FT.				
DEFLECTION CRITERIA							
ROOF TRUSSES* ROOF RAFTERS ROOF RAFTERS (W/O CLG) FLOOR TRUSSES/ BEAMS **	LL/360 LL/180 LL/360 LL/360	TL/240 TL/120 TL/240 TL/240	COMMENTS:				

GUARDRAIL IN-FILL COMPONENTS 50 (LBS)(f) | SHALL BE DESIGNED TO WITHSTAND STAIRS / NON SLEEPING ROOMS | 40 (PSF) | A HORIZONTALLY APPLIED NORMAL

SHEET INDEX

COVER SHEET STRUCTURAL

NOTES, CODE COMPLIANCE,

****TL MAX 1/4" DIFFERENTIAL BETWEEN

	SPECS AND WIND PRESSURES
1	SITE PLAN
2	FOUNDATION PLAN
2.1	FOOTING DETAILS
3	FLOOR PLAN NOTED
4	FLOOR PLAN DIMENSIONED AND LINTEL PLAN
5	EXTERIOR ELEVATIONS
6	ROOF PLAN
7	ROOF FRAMING PLAN
8	ELECTRICAL PLAN
9	SECTIONS AND DETAILS
10	STRUCTURAL SECTION
11	WATERPROOFING DETAILS
12	LINTEL LOADING TABLES AND

CONCRETE DETAILS

13 CONNECTOR SCHEDULE AND

ENGINEERING DETAILS

WIND LOADING CRITERIA ASCE 7-1 WIND SPEED (ULTIMATE) 140.0 MPH WIND SPEED (ALLOWABLE 108.0 MPH

IS 15FT, AND FOR 2 STORY IS 30 FEET

EXPOSURE CATEGORY

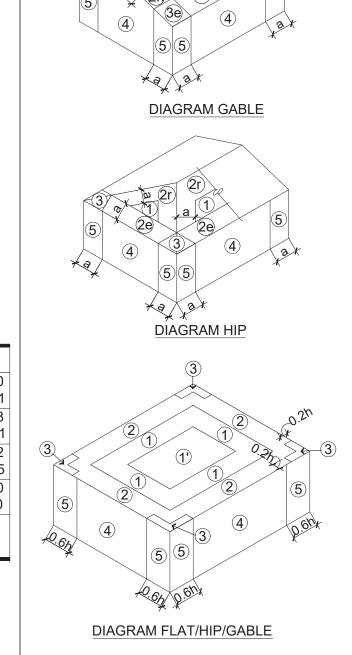
BUILDING CATEGORY

ENCLOSURE CLASSIFICATION ENCLOSED INTERNAL PRESSURE COEFFICIENT +/- 0.18 NOTE: MEAN ROOF HEIGHT FOR TYPICAL SINGLE STORY BUILDING

ASCE 7-16 WALL DESIGN ALLOWABLE COMPONENT AND CLADDING WIND PRESSURES AND SUCTIONS FOR MEAN ROOF HEIGHT ≤ 60 ft

	EFFECTIVE WIND AREA (SQ FEET)		WIND PRESSURE AND SUCTION (PSF) (+) VALUE DENOTES PRESSURE (-) VALUE DENOTES SUCTION						
	AREA		4	5					
	10 - 19.99	1	(+) 49.4 (-) 53.5	2) (+) 61.3 (-) 82.0					
-LS	20 - 49.99	3	(+) 47.2 (-) 51.4	(+) 58.5 (-) 76.4					
WALLS	50 - 99.99	(5)	(+) 44.2 (-) 48.4	6 (+) 54.9 (-) 69.1					
	> 100	7	(+) 42.0 (-) 46.2	8 (+) 52.1 (-) 63.7					
	GARA	ιGE	DOORS*	SOFFIT					
	9'-0" x 7'-0"		16'-0" x 7'-0"						
	(+) 31.1 (-) 35.0	9	(+) 29.8 (-) 33.1	(+) 35.6 (-) 47.0					

	(-) 35.0		(-) 33.1		(-	47.0				
ЭF	AREA		1	(1')		2		(3
ROOF	10 - 19.99	11	(+) 20.0 (-) 78.7	(16) ` '	20.0 45.2	19	` '	20.0)3.7	23)	(+)
BLE	20 - 49.99	12	(+) 18.8 (-) 73.5	(16) ' ') 18.8 45.2	20	(+) 1 (-) 9		24)	(+) (-)
AT/HIP/GABI 'O 7 DEGRE	50 - 99.99	13	(+) 17.2 (-) 66.6	(1 /) ` ') 17.2 45.2	21)	(+) 1 (-) 8		25)	(+) (-)
T/HIF 7 D	> 100	14)	(+) 16.0 (-) 61.46	1101 ') 16.0 45.2	22	(+) 1 (-) 8	6.0 1.6	26)	(+) (-)
FLA ⁻										



/ 21	AREA	1	2e	2n	2r	3e	3r
2	10 - 19.99	②7 (+) 30.0 (-) 70.3	③1) (+) 30.0 (-) 70.3	(+) 30.0 (-) 112.1	③9 (+) 30.0 (-) 112.1	(+) 30.0 (-) 112.1	(+) 30.0 (-) 144.5
7 20	20 - 49.99	②8 (+) 30.0 (-) 70.3	(+) 30.0 (-) 70.3	(+) 30.0 (-) 98.1	(+) 30.0 (-) 98.1	(+) 30.0 (-) 98.1	(+) 30.0 (-) 117.9
5	50 - 99.99	② (+) 30.0 (-) 59.6	③3 (+) 30.0 (-) 59.6	(+) 30.0 (-) 79.8	(+) 30.0 (-) 79.8	(+) 30.0 (-) 79.8	(+) 30.0 (-) 82.9
7 H	> 100	③0 (+) 30.0 (-) 51.7	③4 (+) 30.0 (-) 51.7	③8 (+) 30.0 (-) 65.9	(+) 30.0 (-) 65.9	(+) 30.0 (-) 65.9	(+) 30.0 (-) 82.9
DEGRE							
ţ	AREA	1	(2e)	2n	2r	3e	3r
)		(+) 45 2	(+) 45 2	(+) 45 2	(+) 45 2	(+) 45 2	(+) 45 2

4	AREA	1	(2e)	2n	2r	3e	3r	
7 TO	10 - 19.99	(+) 45.2 (-) 82.9	(+) 45.2 (-) 82.9	60 (+) 45.2 (-) 91.1	(+) 45.2 (-) 82.9	(+) 45.2 (-) 128.4	72 (+) 45.2 (-) 91.1	
: > 27	20 - 49.99	(+) 40.2 (-) 70.3	(+) 40.2 (-) 70.3	(+) 40.2 (-) 81.5	65 (+) 40.2 (-) 70.3	(+) 40.2 (-) 113.8	(+) 40.2 (-) 81.5	
ROOF E	50 - 99.99	(+) 33.5 (-) 53.6	(+) 33.5 (-) 53.6	(+) 33.5 (-) 68.7	(+) 33.5 (-) 53.6	70 (+) 33.5 (-) 94.5	(+) 33.5 (-) 68.7	
LE R REE	> 100	(+) 28.4 (-) 41.0	(+) 28.4 (-) 41.0	(+) 28.4 (-) 59.1	67 (+) 28.4 (-) 41.0	(+) 28.4 (-) 79.8	(+) 28.4 (-) 59.1	
GABI DEGI								

	AREA	1	(2e)	2r	3e	
) 27	10 - 19.99	76 (+) 36.8 (-) 66.1	(+) 36.8 (-) 91.1	(+) 36.8 (-) 91.1	(+) 36.8 (-) 91.1	
20 TO	20 - 49.99	(+) 31.8 (-) 58.5	(+) 31.8 (-) 81.5	(+) 31.8 (-) 81.5	(+) 31.8 (-) 81.5	
X	50 - 99.99	(+) 25.1 (-) 48.6	(+) 25.1 (-) 68.7	(+) 25.1 (-) 68.7	(+) 25.1 (-) 68.7	
IP ROOI EGREE	> 100	79 (+) 20.0 (-) 41.0	(+) 20.0 (-) 59.1	(+) 20.0 (-) 59.1	(+) 20.0 (-) 59.1	
HIP F						
	1551	4	(20)	0.,	20	

HP						
	AREA	1	(2e)	2r	3e	
) 45	10 - 19.99	92 (+) 34.9 (-) 70.3	96 (+) 34.9 (-) 84.0	(+) 34.9 (-) 114.2	(+) 34.9 (-) 111.6	
27 TO 45	20 - 49.99	93 (+) 30.4 (-) 62.6	97 (+) 30.4 (-) 66.5	(+) 30.4 (-) 94.6	(+) 30.4 (-) 84.8	
F > 2	50 - 99.99	(+) 24.5 (-) 52.4	(+) 24.5 (-) 43.4	(+) 24.5 (-) 68.9	(+) 24.5 (-) 49.4	
ОШΙ	> 100	95 (+) 20.0 (-) 44.5	(+) 20.0 (-) 41.0	(+) 20.0 (-) 49.4	(+) 20.0 (-) 49.4	
IP RO(EGREI					-1	•

GENERAL PRESSURE NOTES

1. ABOVE SHOWN PRESSURES ARE ULTIMATE WIND

PRESSURES TO BE APPLIED AS REQUIRED. 2. "a" = END ZONE IS ONLY WITHIN 6'-0" OF ALL EXTERIOR BUILDING CORNERS. INDICATED PRESSURES CAN BE INTERPOLATED FOR OTHER DOOR SIZES.

OTHERWISE USE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREAS. 3. DESIGNATED AREAS WHERE THE ULTIMATE WIND SPEED IS 140 MPH OR GREATER AND IS CONSIDER TO BE IN THE WIND-BOURNE DEBRIS AREA. CONTRACTOR TO PROVIDED ADDITIONAL INFO AS REQUIRED FOR PERMITTING.

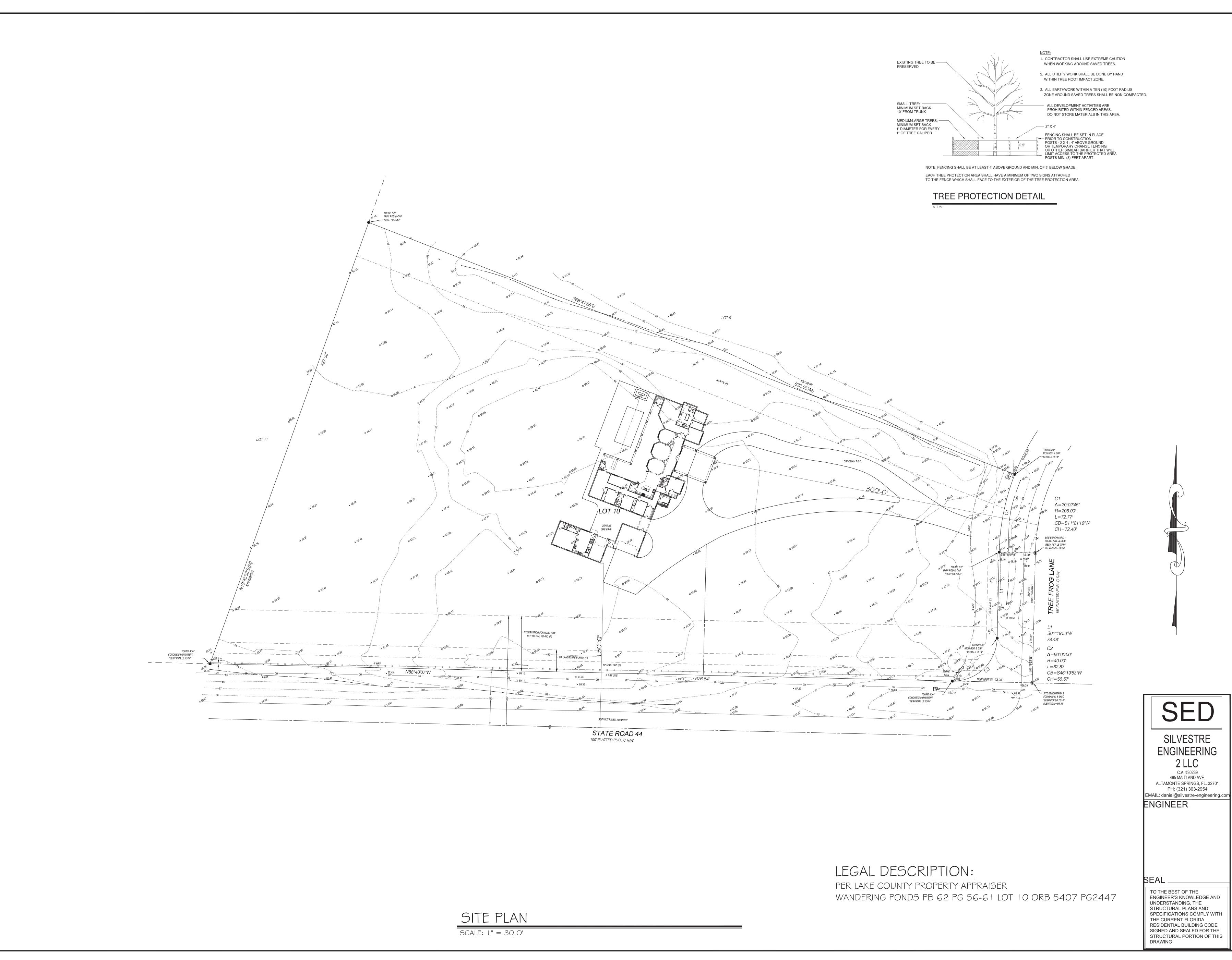
465 MAITLAND AVE.

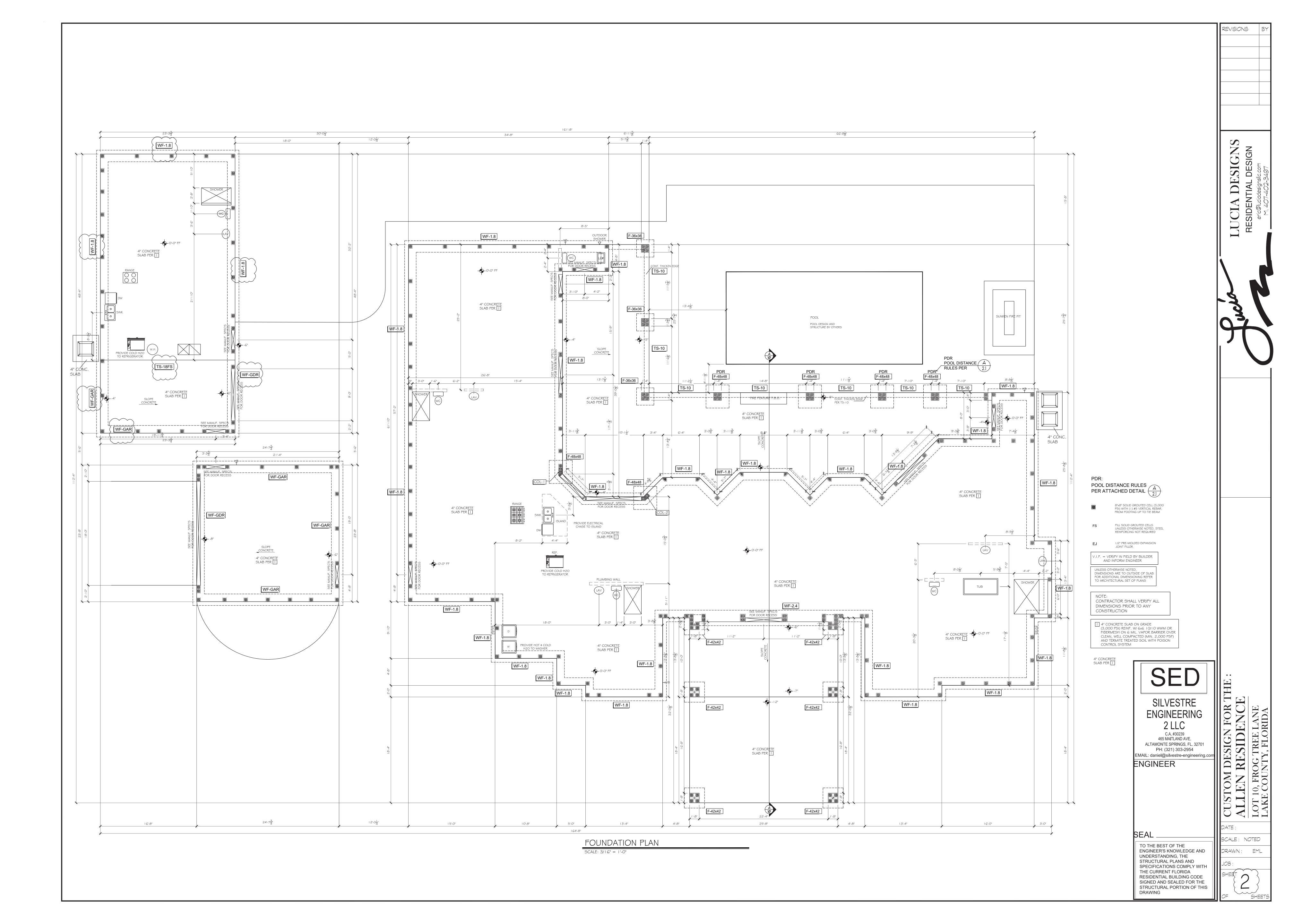
ALTAMONTE SPRINGS, FL. 32701 PH: (321) 303-2954 MAIL: daniel@silvestre-engineering.co

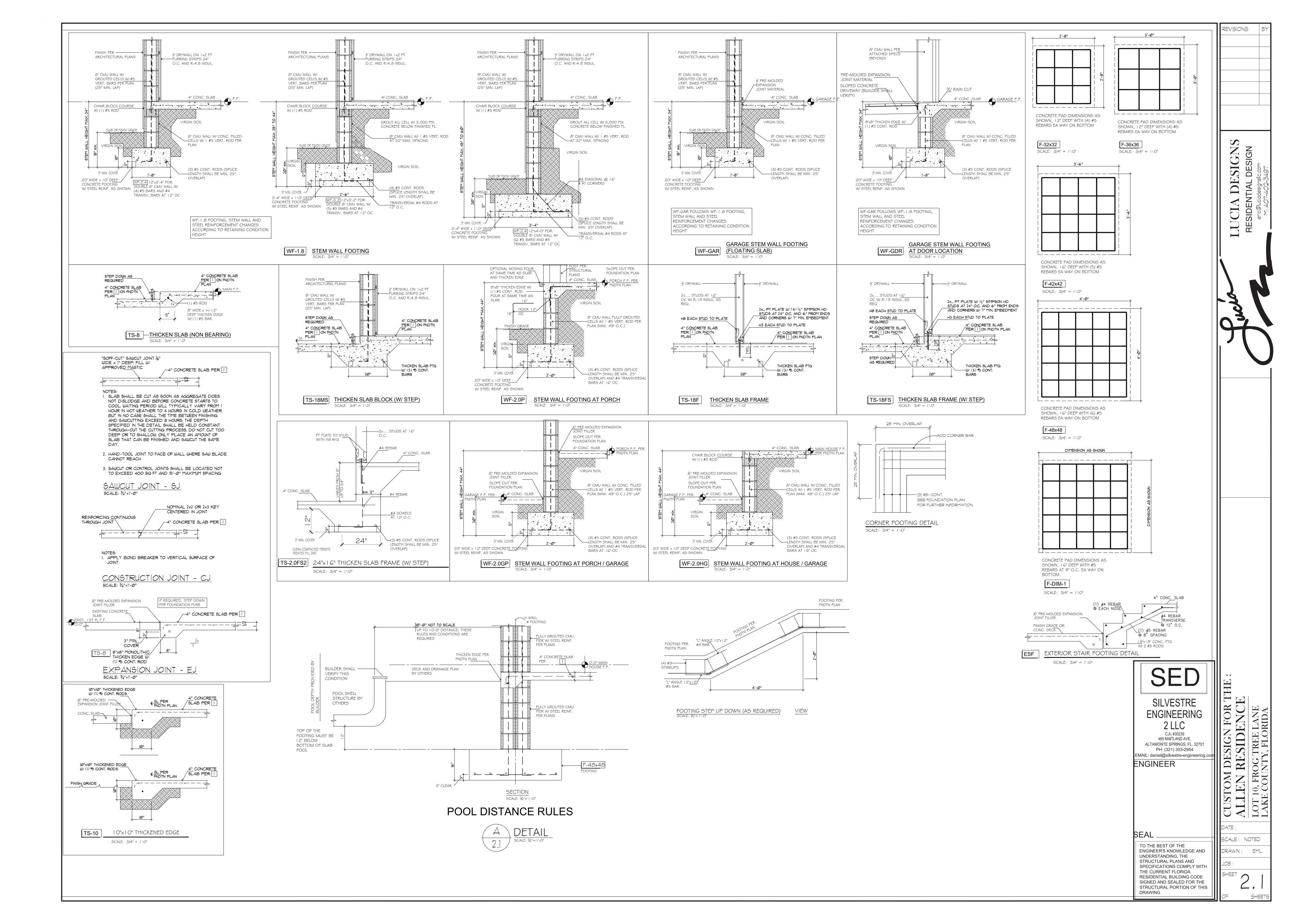
TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND UNDERSTANDING, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE CURRENT FLORIDA RESIDENTIAL BUILDING CODE SIGNED AND SEALED FOR THE STRUCTURAL PORTION OF THIS DRAWING

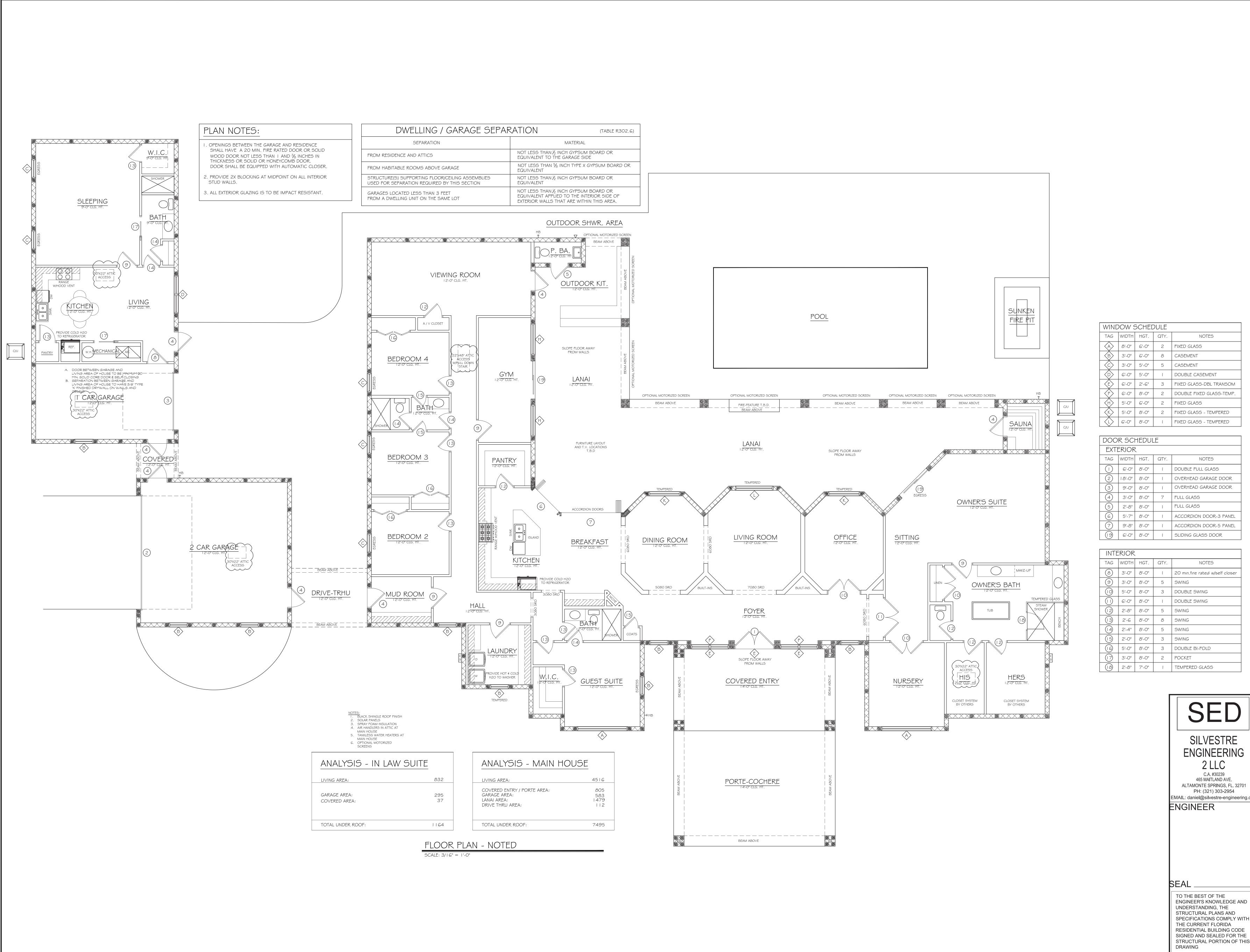
REVISIONS

SCALE: NOTED DRAWN: EML







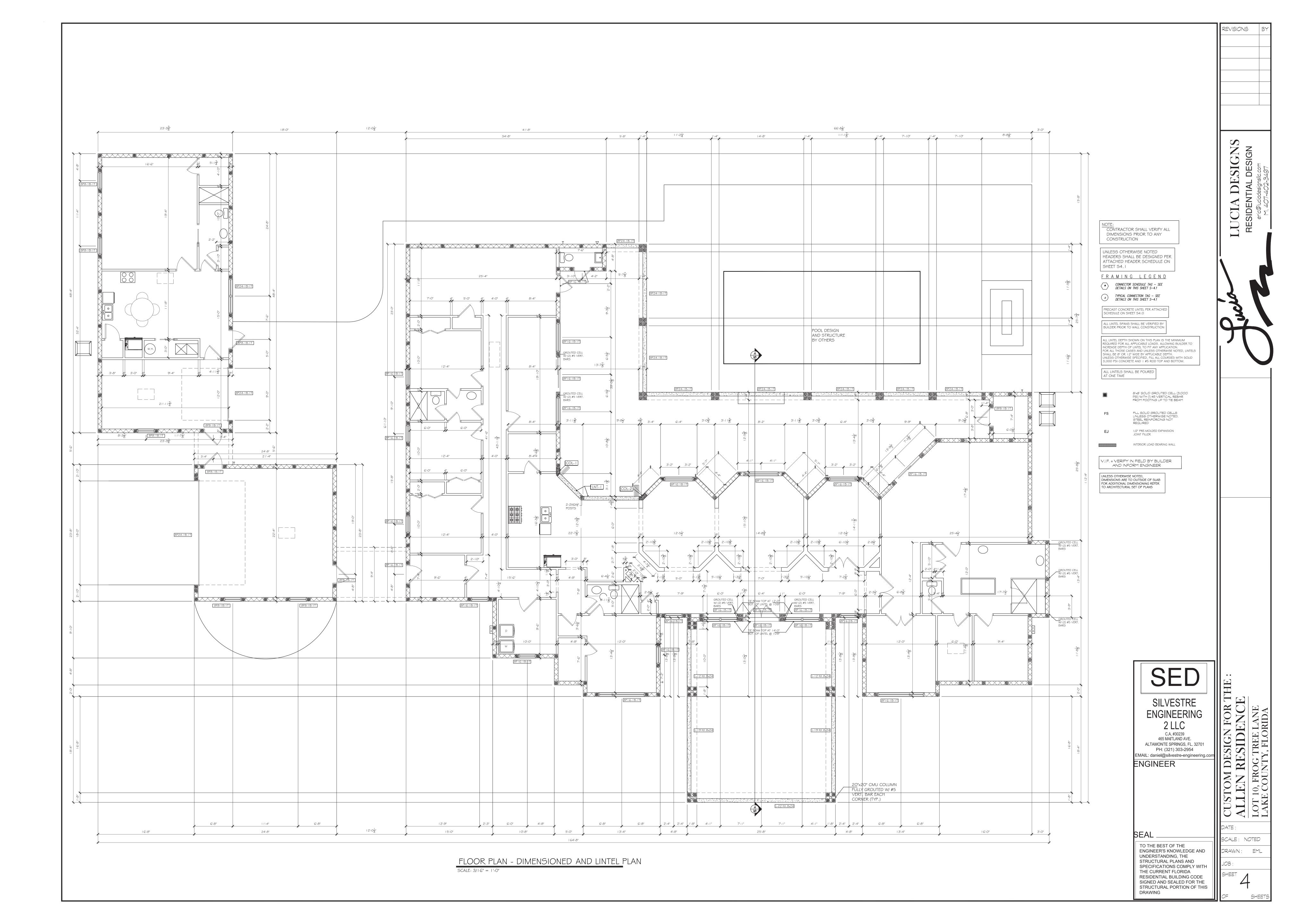


	R SCI	-IFDI II	_	
VTF		ILDUL	_E	
-/\ L	ERIOR	`		
4G	WIDTH	HGT.	QTY.	NOTES
	6'-0"	8'-0"	I	DOUBLE FULL GLASS
2)	18'-0"	8'-0"	I	OVERHEAD GARAGE DOOR
3)	9'-0"	8'-0"	I	OVERHEAD GARAGE DOOR
4)	3'-0"	8'-0"	7	FULL GLASS
5)	2'-8"	8'-0"	I	FULL GLASS
9	5'-7"	8'-0"	-	ACCORDION DOOR-3 PANE
7)	9'-8"	8'-0"	ı	ACCORDION DOOR-5 PANE
9	6'-0"	8'-0"		SLIDING GLASS DOOR
	3) 4) 5) 9	1) 6'-0" 2) 18'-0" 3) 9'-0" 4) 3'-0" 5) 2'-8" 6) 5'-7" 7) 9'-8"		

INTE	RIOR			
TAG	WIDTH	HGT.	QTY.	NOTES
8	3'-0"	8'-0"	I	20 min.fire rated w/self clo
(9)	3'-0"	8'-0"	5	SWING
(9)	5'-0"	8'-0"	3	DOUBLE SWING
	6'-0"	8'-0"	I	DOUBLE SWING
(2)	2'-8"	8'-0"	5	SWING
(3)	2'-6	8'-0"	8	SWING
(14)	2'-4"	8'-0"	5	SWING
(5)	2'-0"	8'-0"	3	SWING
(9)	5'-0"	8'-0"	3	DOUBLE BI-FOLD
(7)	3'-0"	8'-0"	2	POCKET
(18)	2'-8"	7'-0"	ı	TEMPERED GLASS

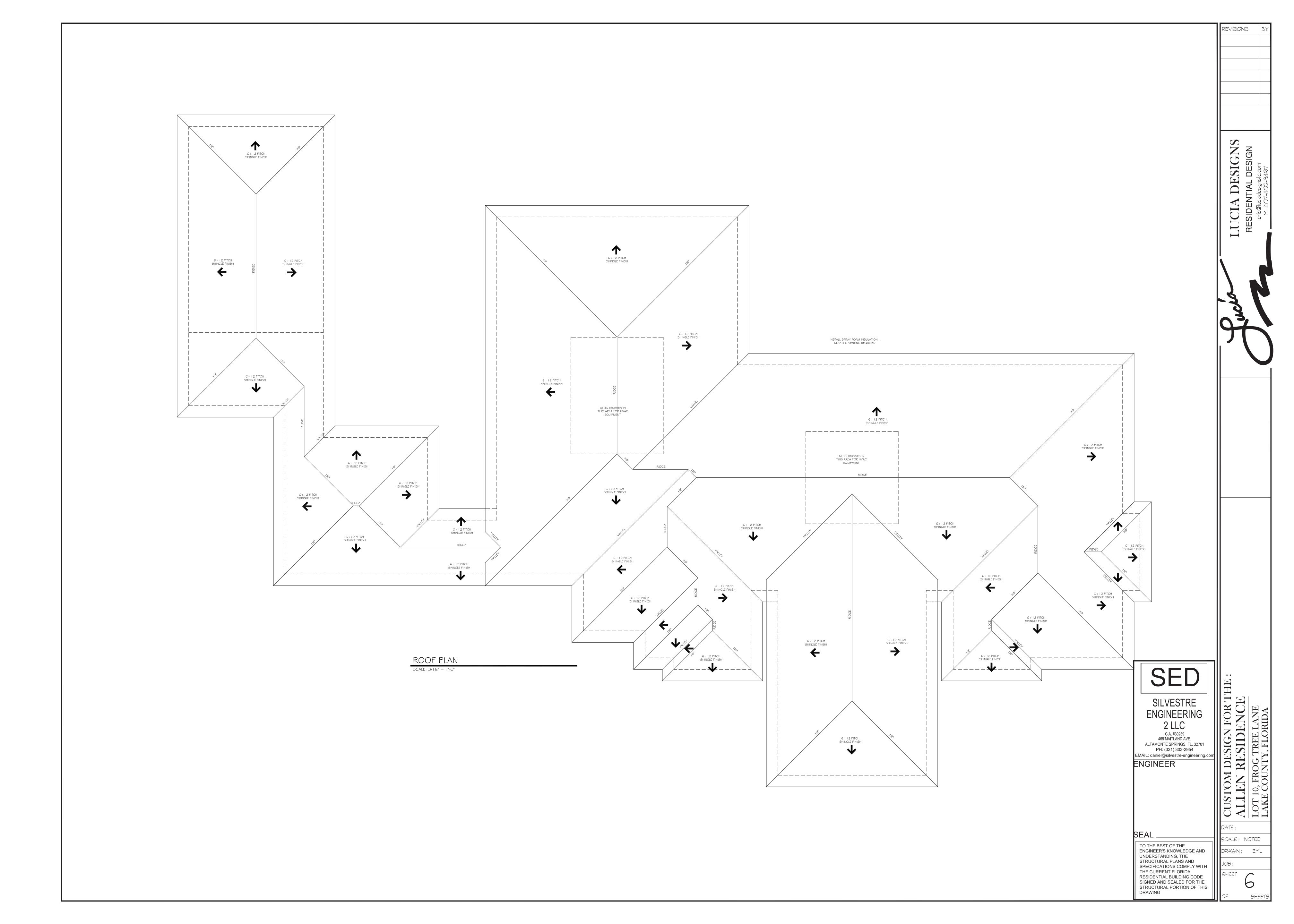
SILVESTRE **ENGINEERING**

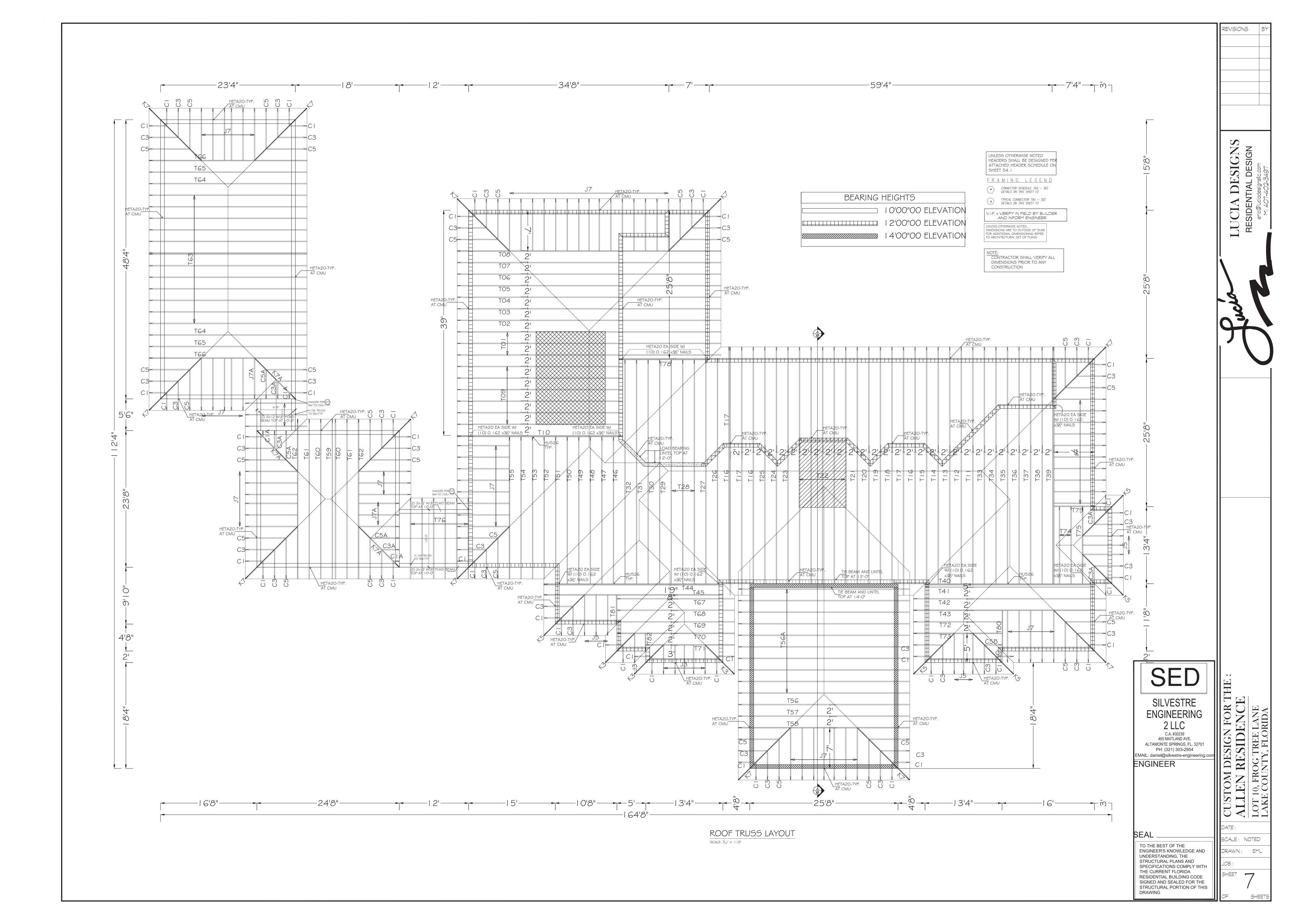
ENGINEER'S KNOWLEDGE AND UNDERSTANDING, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE CURRENT FLORIDA RESIDENTIAL BUILDING CODE SIGNED AND SEALED FOR THE STRUCTURAL PORTION OF THIS

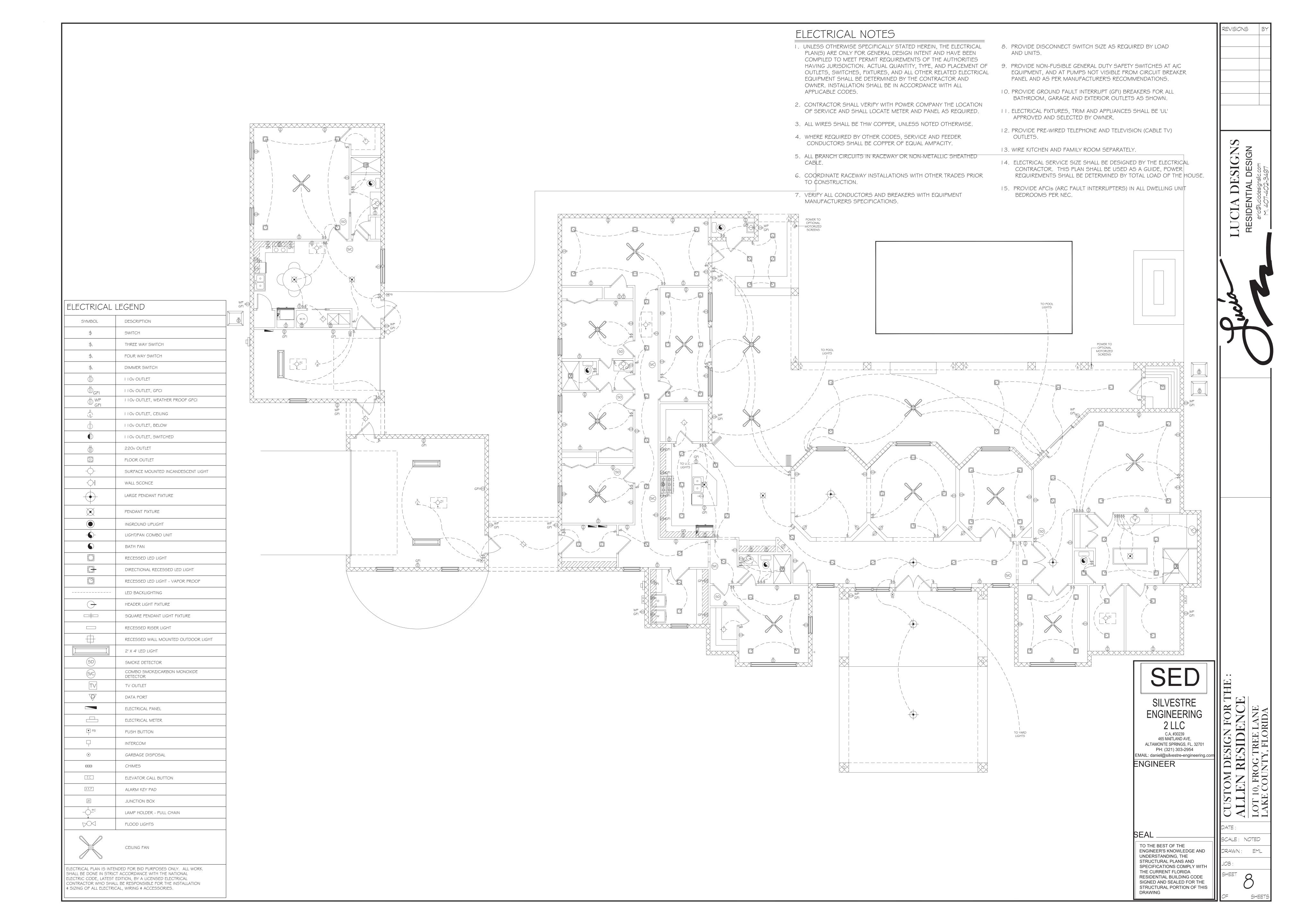


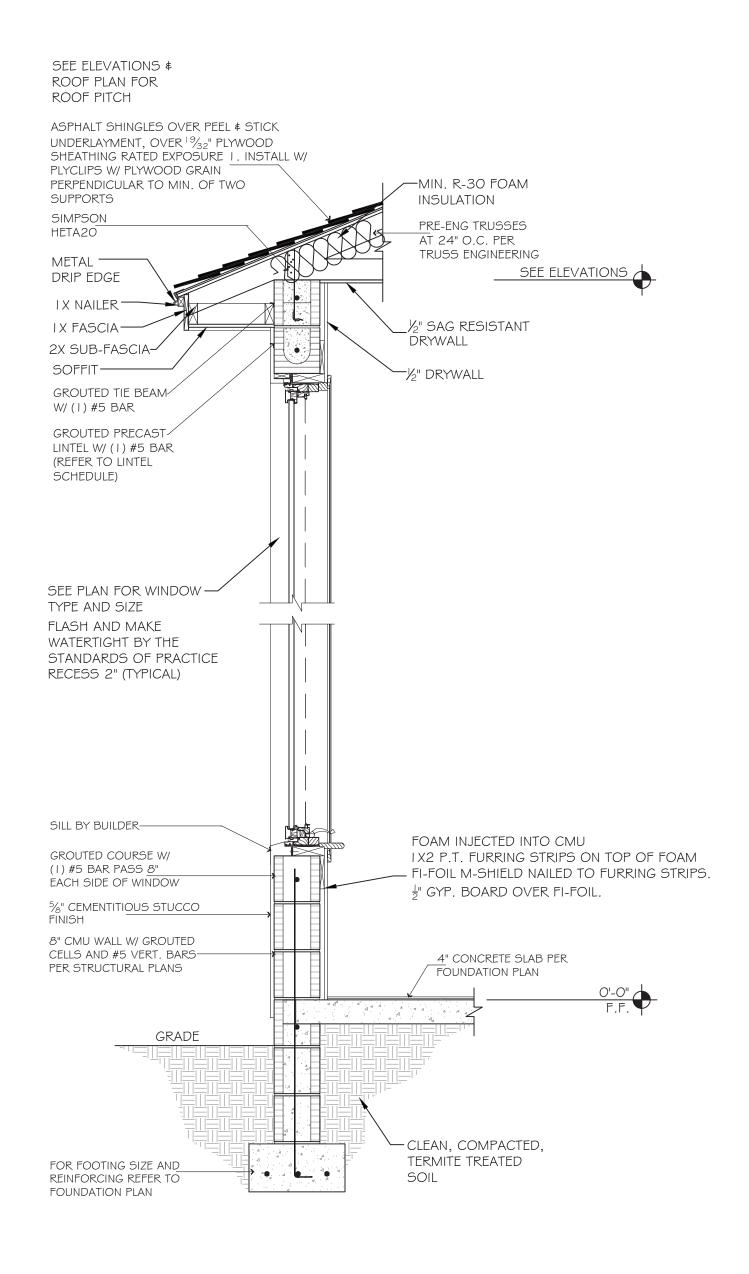


SCALE: NOTED DRAWN: EML

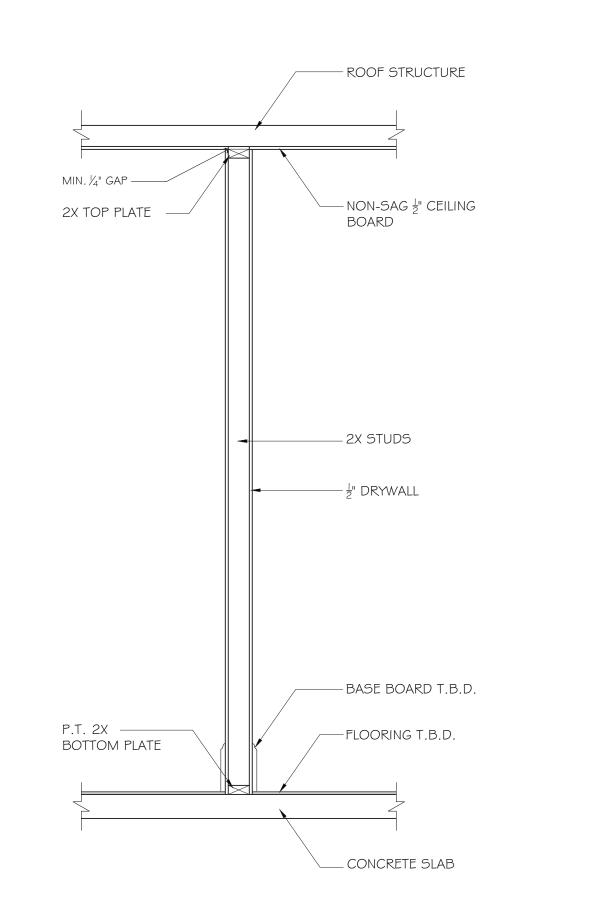




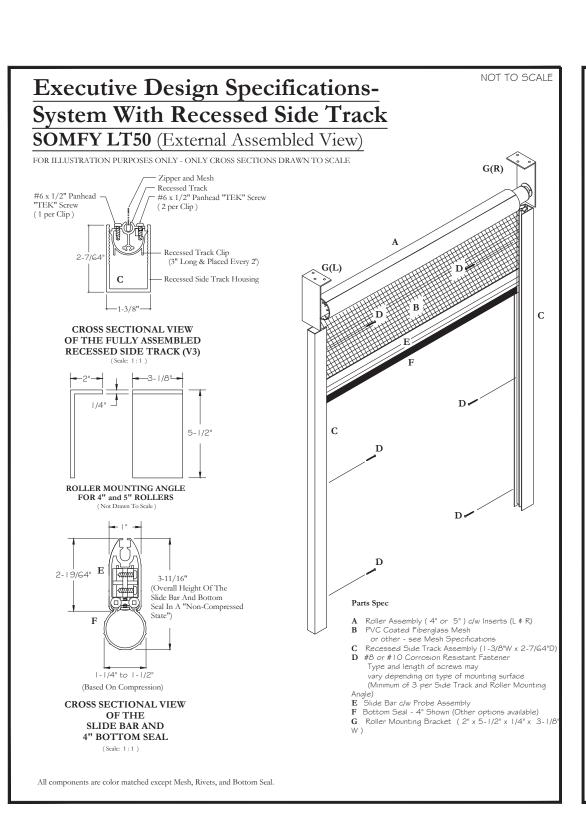




TYPICAL SINGLE STORY WALL SECTION SCALE: 3/4" = 1'-0"



CHANNEL DRAIN DETAILS INTERIOR NON-BEARING WALL SCALE: | " = | '-0"SCALE: 3/4" = 1'-0"



SIDE JAMB

<u>HEAD</u>

FLUSH POOL

CHANNEL -

POOL DECK SLAB —

CHANNEL —

DRAIN

DRAIN

DECK SLAB

PREPARATION OF WINDOW OPENING:

- INSTALL PRESSURE TREATED WOOD BUCK TO PERIMETER OF OPENING USING TAPCON 3/16" X 3 1/2" OR EQUAL PROVIDING 500 LBS. SHEAR STRENGTH WITHIN 6" FROM CORNERS \$ 16" ON CENTER.

- APPLY A CONTINUOUS BEAD OF CAULKING TO SEAL WOOD BUCK TO MAGONING OF THE PROPERTY OF THE PROPER

TO MASONRY OPENINGS. - ENSURE THAT A CLEARANCE OF 1/4" PER SIDE IS LEFT FOR SHIMMING

- FILL CELLS WITH CONCRETE AND REBAR AS REQUIRED BY LOCAL CODE AND POUR SILL BLOCK SMOOTHLY FOR ATTACHMENT OF SILL BUCK (BY OTHERS).

INSTALLATION OF WINDOW:

- REFER TO INSTALLATION INSTRUCTIONS FOR THE SPECIFIC PRODUCT BEING INSTALLED.

- SET WINDOW IN OPENING, SHIMMING, LEVELING AND SQUARING TO ENSURE PROPER OPERATION.

- INSTALL # 8 WAFER HEAD OR S.M.S. I I/4" OR WOOD SCREW THROUGH ALL PREDRILLED HOLES IN THE INSTALLATION FIN TO SECURE UNIT.

- ENSURE THAT THE INSTALLATION FIN IS SEALED TO THE WOOD 2 X BUCK WITH A CONTINUOUS BEAD OF CAULKING.

- FILL VOID BETWEEN WINDOW AND BUCK WITH INSULATION BEING CAREFUL NOT TO BOW THE FRAME (BY OTHERS).

- WATER PROTECT FINS AND MASONRY WITH NPI OR EQUAL. COVERING FROM ALUMINUM CLAD TO MASONRY.

ALUMINUM CLAD TO MASONRY.
- WATERPROTECT MASONRY SILL AND UP SIDES 6" WITH SELF LEVELING

- LEAVE 1/4" GAP BETWEEN EXTERIOR OF WINDOW AND FINISH MATERIALS FOR CAULK JOINT TO ALLOW FOR MATERIALS EXPANSION.

WINDOW DETAIL

N.T.S. - DRYWALL RETURNS

TILE FINISH

ON MUD BED

STEP DOWN FOOTING PER

E.O.R.

TILE FINISH

ON MUD BED

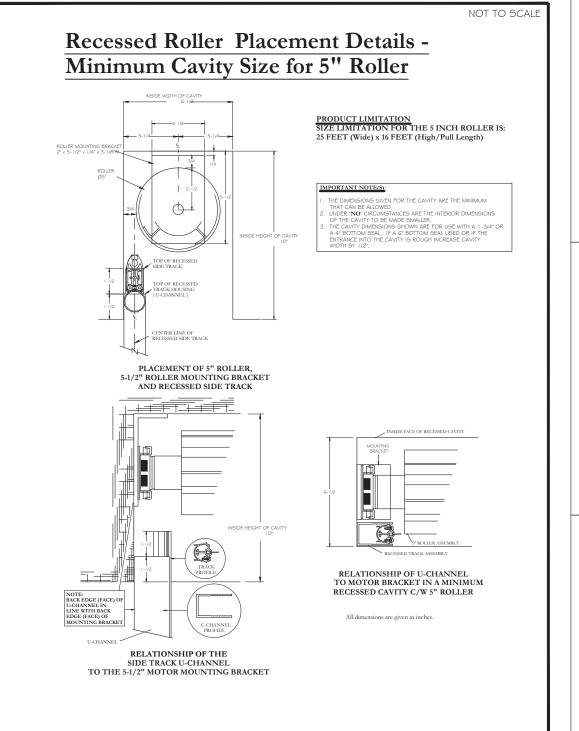
FOOTING PER

E.O.R.

I. ALL DIMENSIONS ARE TO BE FIELD VERIFIED. 2. SEE DRAIN MANUFACTURE FOR INSTALLATION

DETAILS AND DIMENSIONS.

COMPOSITE FRAME WINDOW



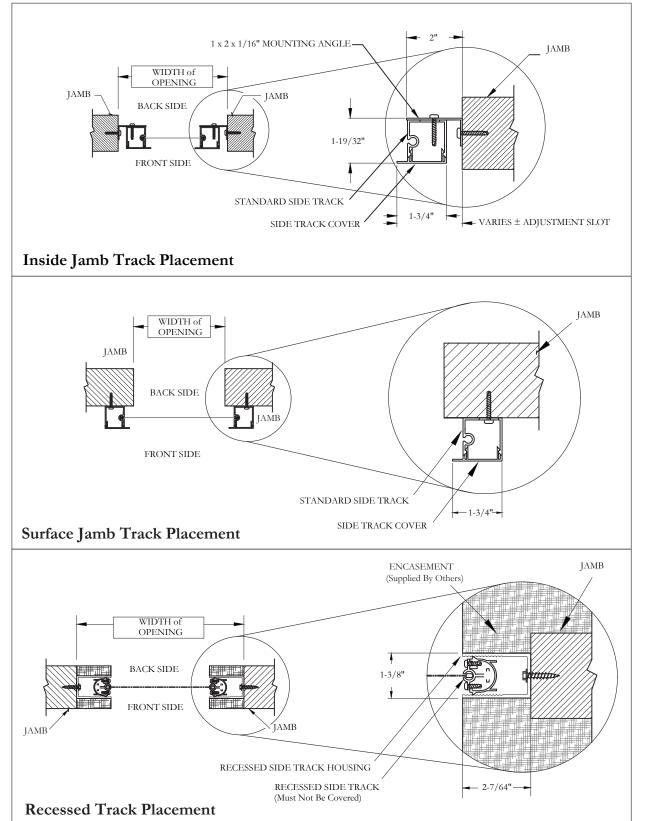
GROUTED CELL WITH (1) #5 VERT. BAR

FROM FOOTING UP

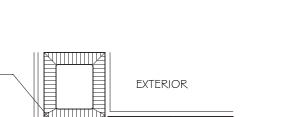
2X TO BE HELD OUT — TO RUN FLUSH WITH

THE FURRING STRIPS

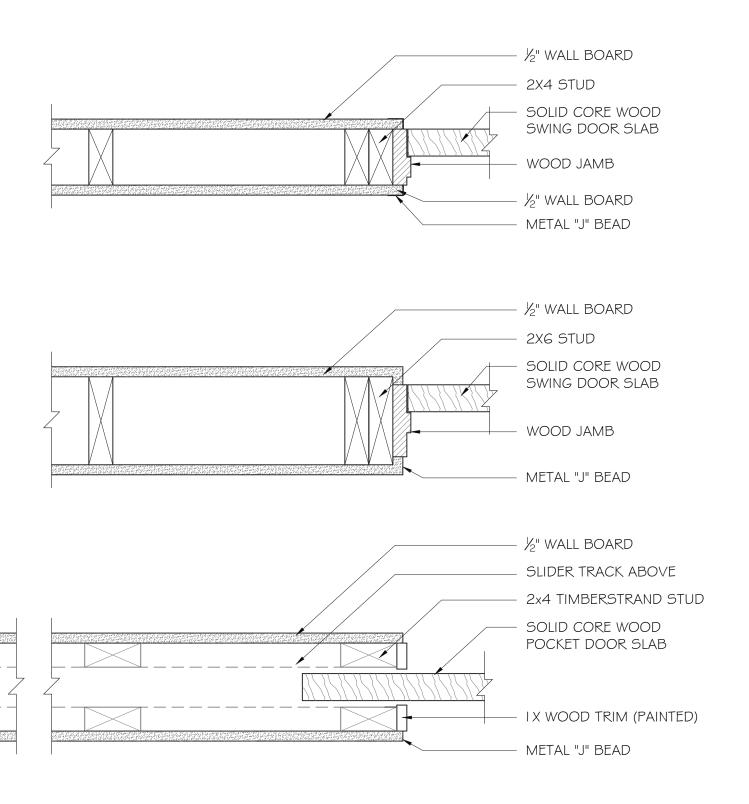
TO TIE BEAM



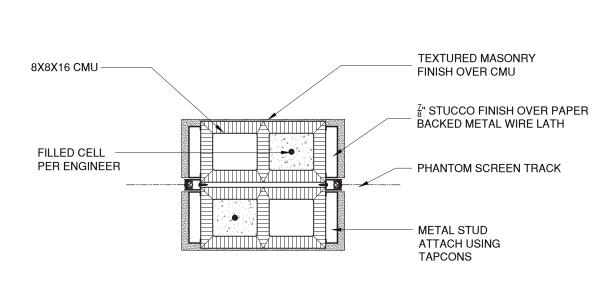
Executive Design Specifications- Side Track Placement



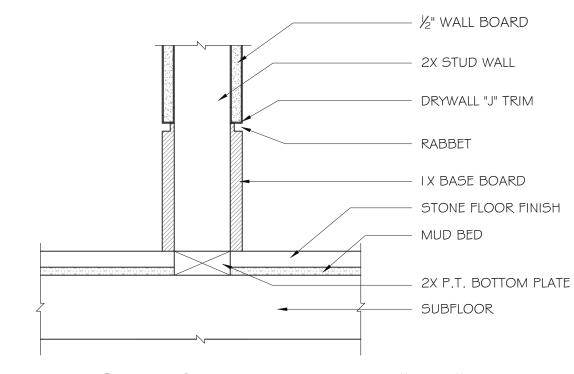






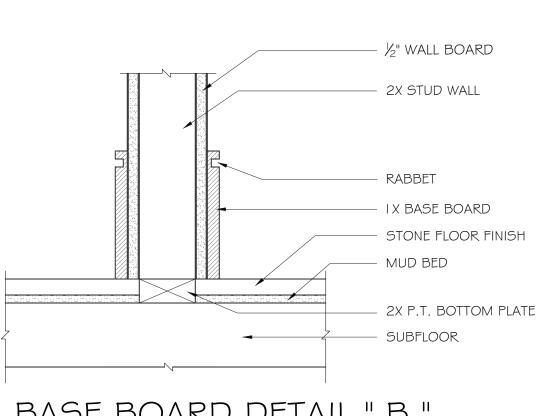


RECESSED TRACK PLACEMENT DETAIL -IN 16" CMU COLUMN W/ STUCCO / T.M.F. SCALE - 1" = 1'-0"



BASE BOARD DETAIL " A "

NOT TO SCALE



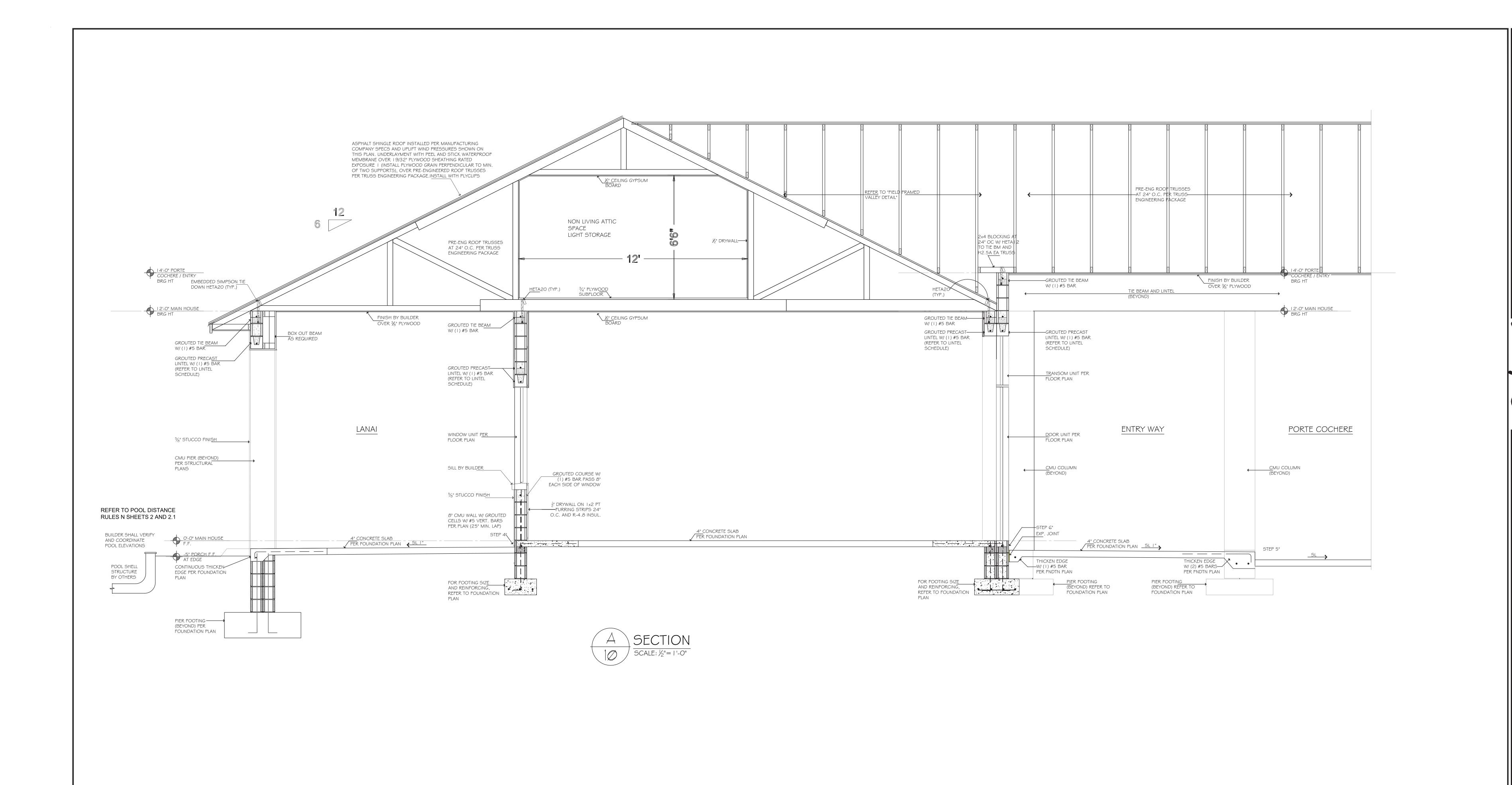
BASE BOARD DETAIL " B " NOT TO SCALE

SILVESTRE **ENGINEERING** C.A. #30239 465 MAITLAND AVE. ALTAMONTE SPRINGS, FL. 32701 PH: (321) 303-2954 MAIL: daniel@silvestre-engineering.c

ENGINEER

SIG DE

SCALE: NOTED TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND UNDERSTANDING, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE CURRENT FLORIDA RESIDENTIAL BUILDING CODE SIGNED AND SEALED FOR THE STRUCTURAL PORTION OF THIS DRAWING



SED

SILVESTRE ENGINEERING

2 LLC

C.A. #30239

465 MAITLAND AVE.

ALTAMONTE SPRINGS, FL. 32701

PH: (321) 303-2954

WAIL: daniel@silvestre-engineering.c

EMAIL: daniel@silvestr

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THE CURRENT FLORIDA
RESIDENTIAL BUILDING CODE
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STRUCTURAL PORTION OF THIS
DRAWING

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ALLEN RESII

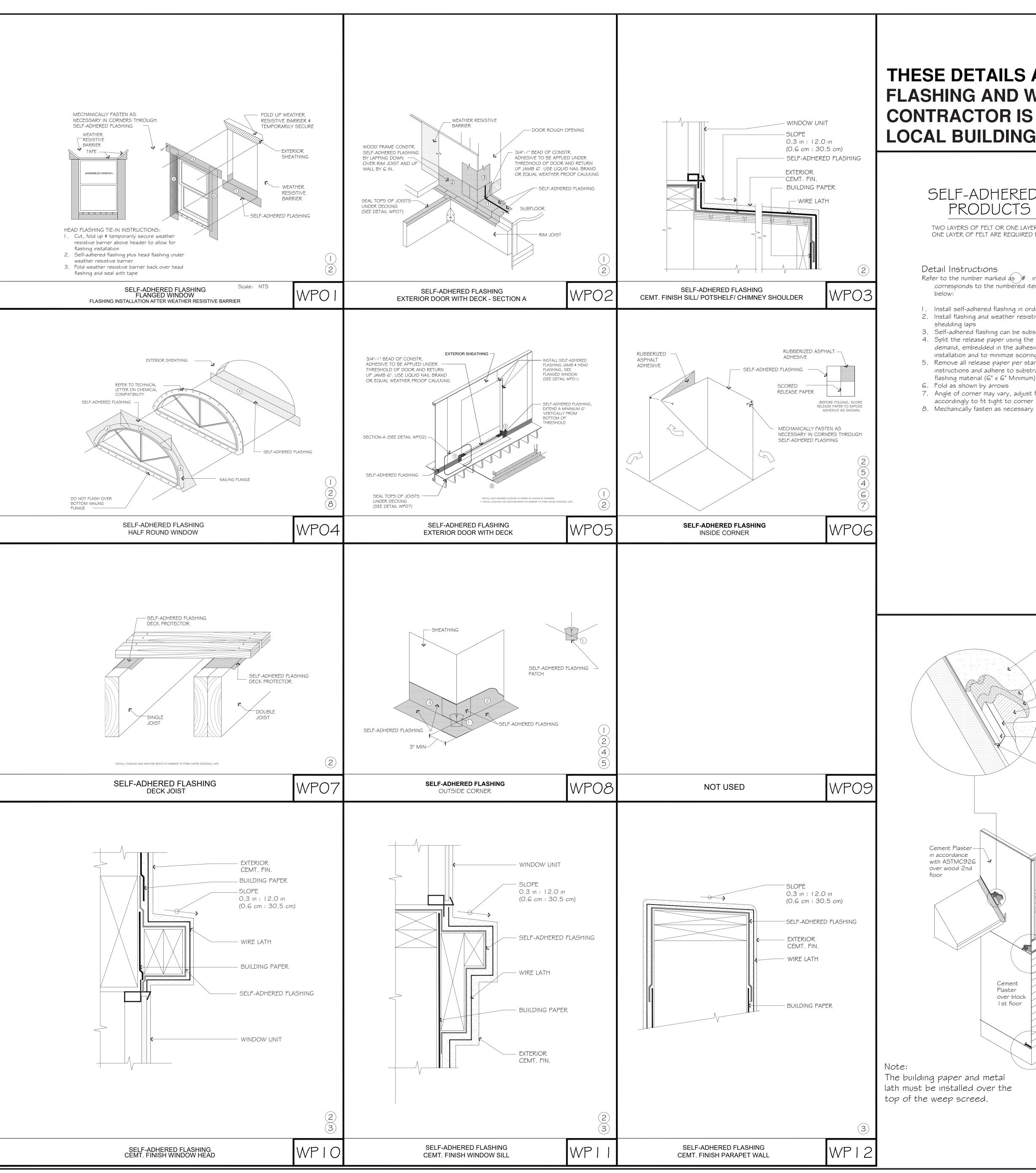
ALLEN RESII

LOT 10, FROG TRE

LAKE COUNTY, FL

REVISIONS

DESIGNS ITIAL DESIGN



THESE DETAILS ARE GENERIC AND MEANT TO SHOW GENERAL FLASHING AND WATERPROOFING METHODS TO BE USED. CONTRACTOR IS TO USE APPROPRIATE TECHNIQUES FOR LOCAL BUILDING PRACTICES AND CLIMATE.

SELF-ADHERED FLASHING PRODUCTS DETAILS

TWO LAYERS OF FELT OR ONE LAYER OF HOUSE WRAP AND ONE LAYER OF FELT ARE REQUIRED BEHIND STUCCO. FBC R703.2.1

Detail Instructions

Refer to the number marked as # in each detail that corresponds to the numbered items in the list of instructions

- I. Install self-adhered flashing in order as shown by numbers
- 2. Install flashing and weather resistive barrier to form water shedding laps
- 3. Self-adhered flashing can be substituted for building paper 4. Split the release paper using the ripcord (Split release on demand, embedded in the adhesive layer) - for ease of installation and to minimize scoring cuts
- 5. Remove all release paper per standard installation instructions and adhere to substrate using a square piece of flashing material (6" x 6" Minimum)
- 6. Fold as shown by arrows 7. Angle of corner may vary, adjust folding of the flashing
- accordingly to fit tight to corner

FLASHING REQUIREMENTS

R703.2 Weather-resistant sheathing paper. One layer of No. 15 asphalt felt, free from holes and breaks, comply with ASTM D 226 for Type 1 felt or other approved water-resistive barrier shall be applied over studs or sheathing of all exterior walls. Such felt or material shall be applied horizontally, with the upper layer lapped over the lower layer not less than 2 inched (51 mm). Where joints occur, felt shall be lapped not less than 6 inches (152 mm). The felt or other approved material shall be continuous to the top of walls and terminated at penetrations and building appendages in a manner to meet the requirements of the exterior wall envelope as described in Section R703.1.

Exception: Omission of the water-resistive barrier is permitted in the following situations:

- 1. In detached accessory buildings.
- 2. Under wall finish materials as permitted in Table R703.4.

R703.8 Flashing. Approved corrosion-resistant flashing shall be applied shingle-fashion in a manner to prevent entry of water into the wall cavity or penetration of water to the building structural framing components. Self-adhered membranes used as flashing shall comply with AAMA 711. The flashing shall extend to the surface of the exterior wall finish. Approved corrosion-resistant flashing shall be installed at all of the following locations:

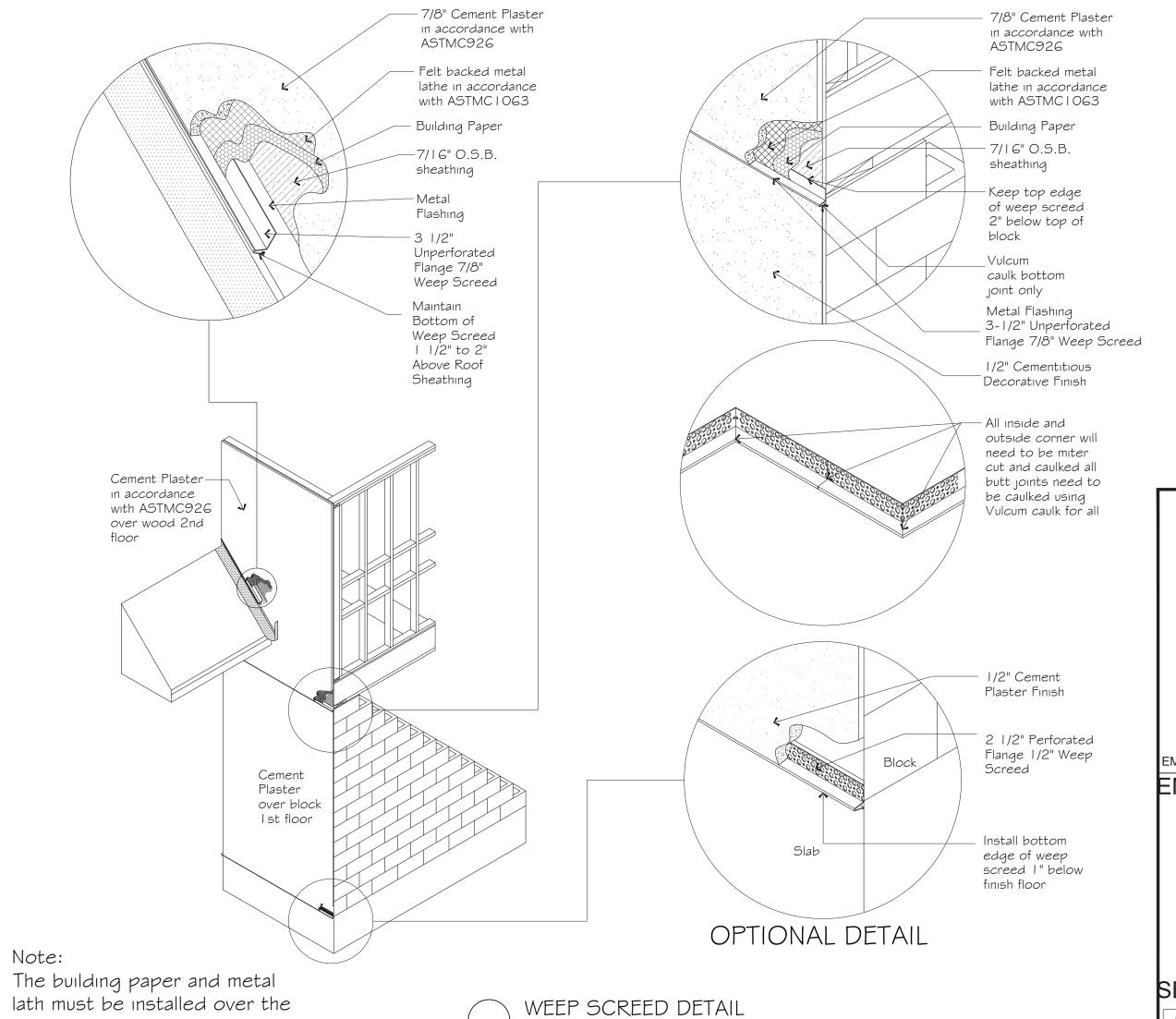
1. Exterior window and door openings. Flashing at exterior window and door openings shall extend to the surface of the exterior wall finish or to the water-resistive barrier for subsequent drainage. Flashing at exterior window and door openings shall be installed in accordance with one or more of the following or other approved

- 1.1 The fenestration manufacturer's written flashing instructions. 1.2 The flashing manufacturer's written installation instructions.
- 1.3 In accordance with FMA/AAMA 100, FMA/AAMA 200, or
- 1.4 In accordance with the flashing method of a registered design professional.
- 2. At the intersection of chimneys or other masonry construction with frame or stucco walls, with projecting lips on both sides under stucco
- 3. Under and at the ends of masonry, wood or metal copings and sills. 4. Continuously above all projecting wood trim.
- 5. Where exterior porches, decks or stairs attach to a wall or floor
- assembly of wood-frame construction. 6. At wall and roof intersections.

FMA/AAMA 250.

7. At built-in gutters.

R703.15 Drained assembly wall over mass assembly wall. Where wood frame or other types of drained wall assemblies are constructed above mass wall assemblies, flashing or other approved drainage system shall be installed as required by R703.8.



SCALE: NOT TO SCALE

SILVESTRE

ENGINEERING C.A. #30239 465 MAITLAND AVE. ALTAMONTE SPRINGS, FL. 32701

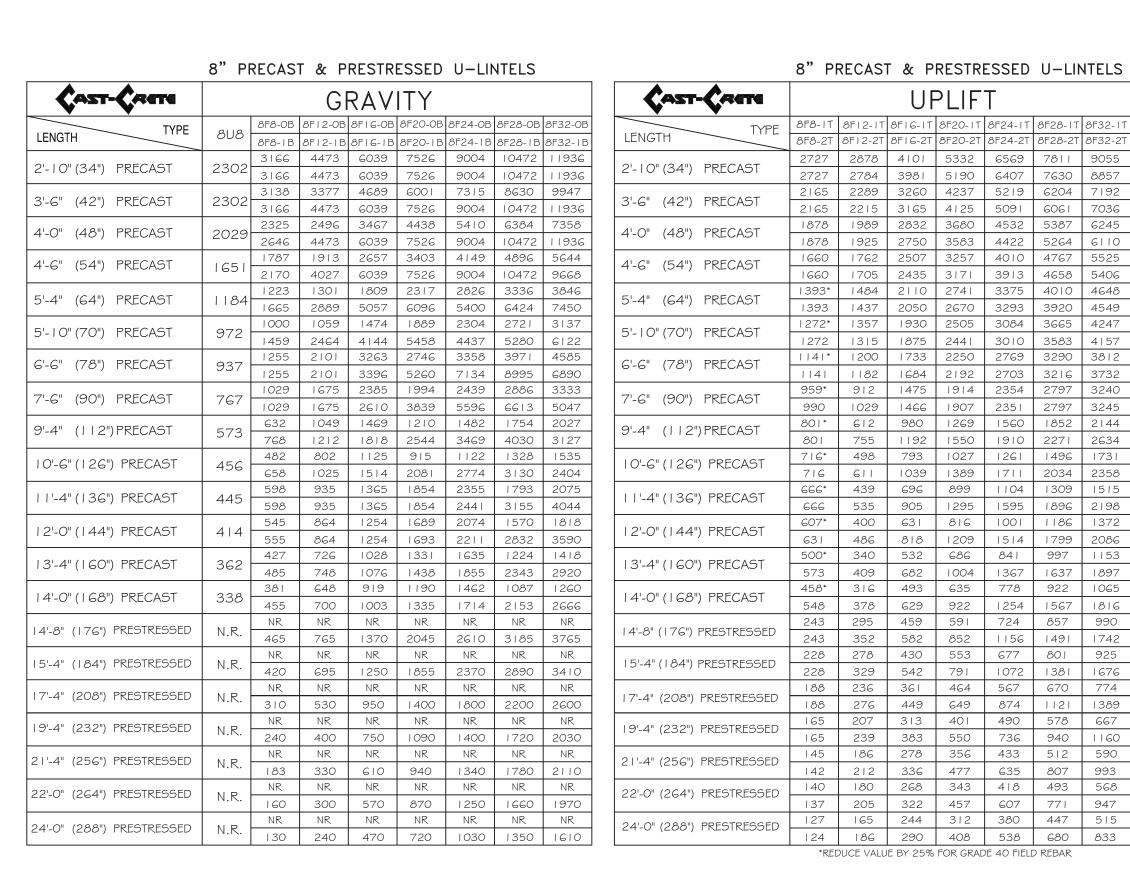
PH: (321) 303-2954 MAIL: daniel@silvestre-engineering.c ENGINEER

DRAWING

TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND UNDERSTANDING, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE CURRENT FLORIDA RESIDENTIAL BUILDING CODE SIGNED AND SEALED FOR THE STRUCTURAL PORTION OF THIS

SCALE: NOTED

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FOR GRAVITY, UPLIFT & LATERAL LOADS

	8" PF	RECAS	ΓW/	2" RE	CESS	DOOR	U-LIN	TELS
CAST-CRETE			GF	RAVI	TY			
TYPE	anuc.	8RF6-0B	8RF10-0B	8RF14-0B	8RF18-0B	8RF22-0B	8RF26-0B	8RF30-01
LENGTH	8RU6	8RF6-1B	8RF10-1B	8RF14-1B	8RF18-1B	8RF22-1B	8RF26-1B	8RF30-11
4'-4" (52") PRECAST	1.480	1591	3053	2982	3954	4929	5904	6880
4-4 (52) TRECAST	1489	1827	3412	4982	6472	7947	9416	10878
4'-6" (54") PRECAST	1257	1449	2782	2714	3600	4487	5375	6264
4-6 (34) TRECAST	1357	1702	3412	4982	6472	7947	9416	10878
	705	832	1602	1550	2058	2566	3075	3585
5'-8" (68") PRECAST	785	1153	2162	4074	6472	6516	5814	6839
FLIOU (701) PRECACT	725	779	1500	1449	1924	2400	2876	3352
5'-10" (70") PRECAST	735	1103	2051	3811	6472	6516	5450	6411
6'-8" (80") PRECAST	800	907	1677	2933	2576	3223	3872	4522
6-0 (00) TRECAST	822	907	1677	2933	4100	6730	8177	6707
ZI CII (QQII) BRECAGI	CCE	761	1377	2252	1958	2451	2944	3439
7'-6" (90") PRECAST	665	764	1377	2329	3609	5492	6624	5132
9'-8" (116") PRECAST	371	420	834	1253	1071	1342	1614	1886

535 | 928 | 1497 | 2179 | 2618 | 3595 | 2875

		8" PF	RECAST	ΓW/	2" RE	CESS	DOOR	U-LIN	TELS	
€ c	NST-CRETE			U	PLIF	T			LAT	ERAL
	TYPE	8RFG-IT	8RF10-1T	8RF14-1T	8RF18-1T	8RF22-1T	8RF26-1T	8RF30-1T	8 DUIC	a DEC
LENGTH		8RF6-2T	8RF10-2T	8RF14-2T	8RF18-2T	8RF22-2T	8RF26-2T	8RF30-2T	8RU6	8RF6
1 1 1 1	52") PRECAST	1244	1573	2413	3260	4112	4967	5825	020	020
4-4 (,	JZ) I NLCAJI	1244	1519	2339	3170	4008	4850	5696	932	932
4'-6" (!	54") PRECAST	1192	1507	2311	3121	3937	4756	5577	853	853
-1-0 (.	5-1) TICONOT	1192	1455	2240	3036	3837	4643	5453	000	
5'-8" ((68") PRECAST	924*	1172	1795	2423	3055	3689	4325	501	501
5-0 (60) FRECASI	924	1132	1741	2357	2978	3603	4230	501	501
5' 10" (70") PRECAST	896*	1138	1742	2352	2965	3581	581 4198	469	469
5-10 (70) INLCASI	896	1099	1690	2288	2891	3497	4106	469	469
C 8 (80") PRECAST	778	882	1513	2042	2573	3107	3642	920	1100
6-0 (OO) TRECAST	778	956	1468	1987	2509	3035	3563	830	1100
7'-6" (9	ann precast	688	697	1325	1810	2280	2753	3227	710	
/-6 (;	90") PRECAST	688	849	1302	1762	2225	2690	3157	710	941
9'-8" (I I 6") PRECAST	533*	433	808	1123	1413	1704	1995	510	614
<i>J-O</i> (TIOTINLUADI	533	527	1009	1369	1728	2088	2450	516	
		*RFI	DUCE VALU	F BY 15%	FOR GRAD	F 40 FIFLE) RFBAR			

8F24-1B/17

ACCEPTABLE AS LONG AS ALL COURSES ABOVE P.C. LINTEL ARE FILLED W/ GROUT.

NOMINAL HEIGHT -

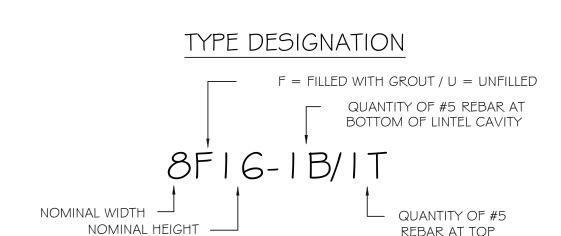
PRE-CAST LINTEL ENGINEERING PER CAST-CRETE.

MATERIALS

- I. f'c precast lintels = 3500 psi.
- 2. fc prestressed lintels = 6000 psi.
- 3. f'c grout = 3000 psi w/ maximum 3/8" aggregate. 4. Concrete masonry units (CMU) per ASTM C90 w/ minimum net area compressive strength = 1900 psi.
- 5. Rebar provided in precast lintel per ASTM A615 GR60.
 Field rebar per ASTM A615 GR40 or GR60.
- 6. Prestressing strand per ASTM A416 grade
- 270 low relaxation. 7. 7/32 wire per ASTM A510. 8. Mortar per ASTM C270 type M or S.

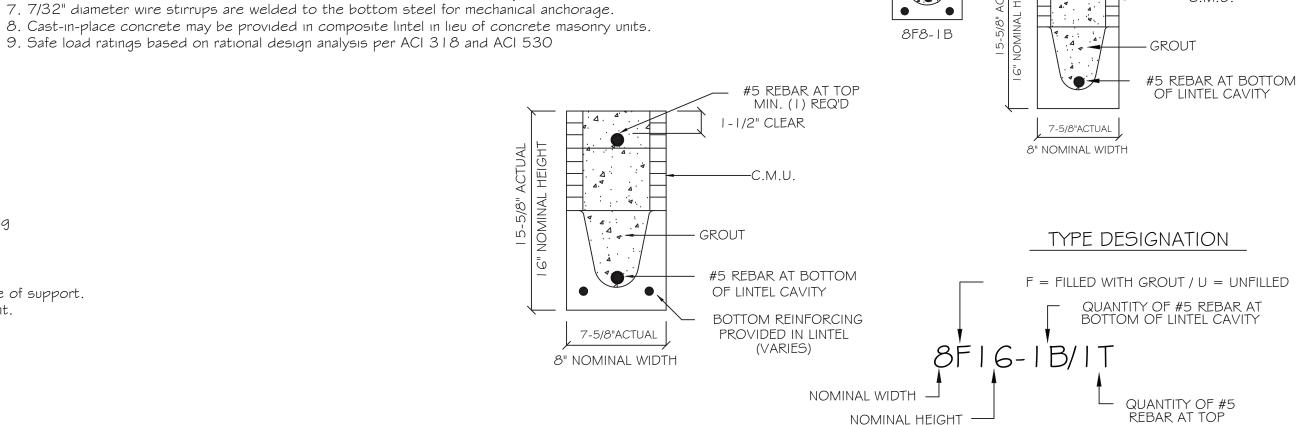
SAFE LOAD TABLE NOTES

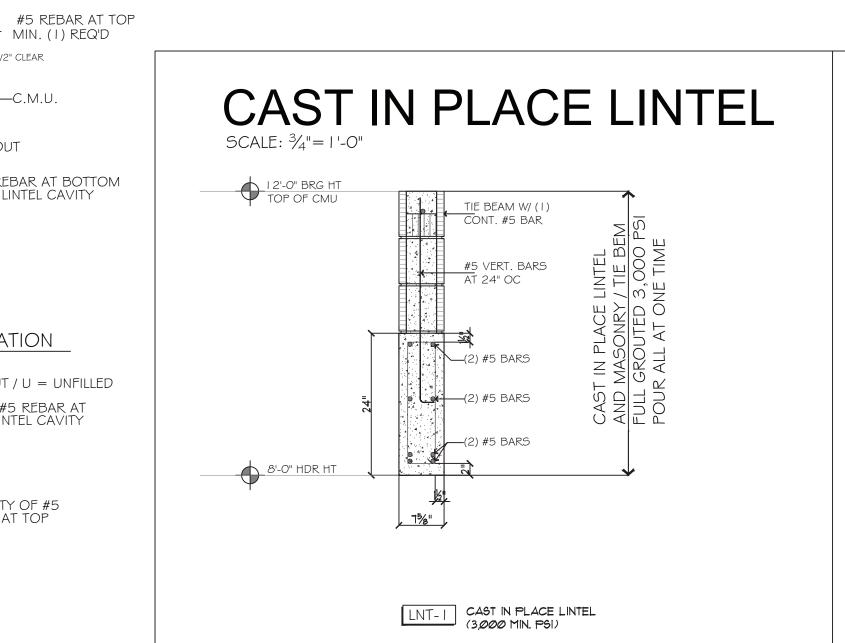
- 1. All values based on minimum 4" bearing. Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6-1/2". Safe loads for all recessed lintels based on 8" nominal bearing. 2. N.R. = Not Rated.
- 3. Safe loads are total superimposed allowable load on the section specified. 4. Safe loads based on grade 40 or grade 60 field rebar.
- 5. Additional lateral load capacity can be obtained by the designer by providing additional reinforced masonry above the precast lintel.
- 6. One #7 rebar may be substituted for two #5 rebars in 8" lintels only. 7. The designer may evaluate concentrated loads from the safe load tables by
- calculating the maximum resisting moment and shear at d-away from the face of support. 8. For composite lintel heights not shown, use safe load from next lower height.
- 9. All safe loads in units of pounds per linear foot.



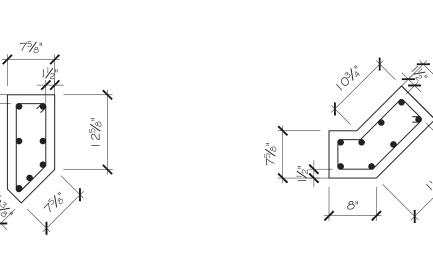


- 2. Shore filled lintels as required. 3. Installation of lintel must comply with the architectural and/or structural drawings. 4. Lintels are manufactured with 5-1/2" long notches at the ends to accommodate
- vertical cell reinforcing and grouting.
 5. All lintels meet or exceed U360 vertical deflection, except lintels 17'-4" and
- longer with a nominal height of 8" meet or exceed L/180.
- 6. Bottom field added rebar to be located at the bottom of the lintel cavity.
- 8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.





CAST IN PLACE COLUMNS



TOTAL OF (7) #5 VERTICAL BARS W/ #3 COL- | CAST IN PLACE COLUMN

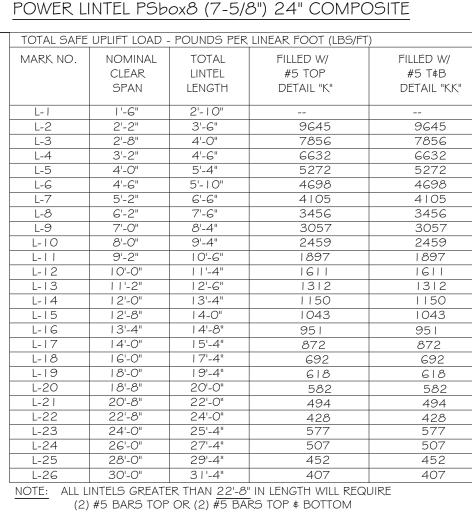
TOTAL OF (8) #5 VERTICAL BARS W/ #3 STIRRUPS AT 6" O.C. COL-2 CAST IN PLACE COLUMN (3,000 MIN. PSI)

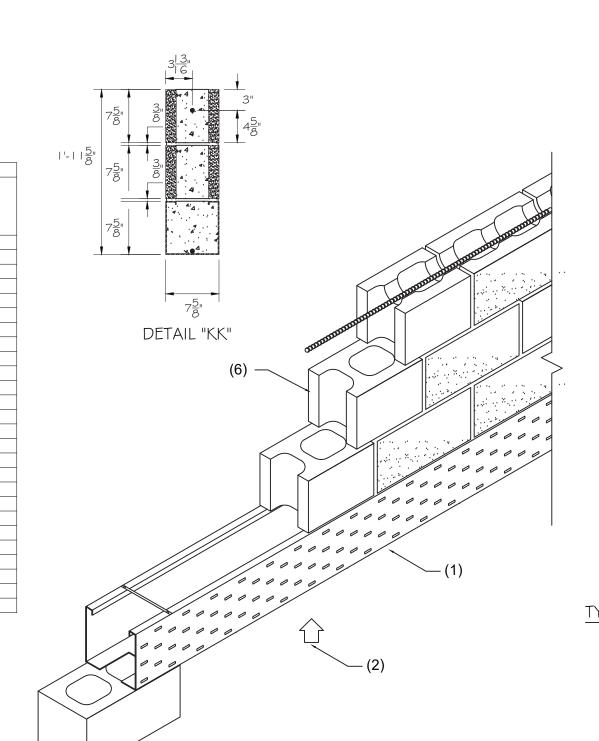
POWER STEEL BOX AND WIRE LINTELS

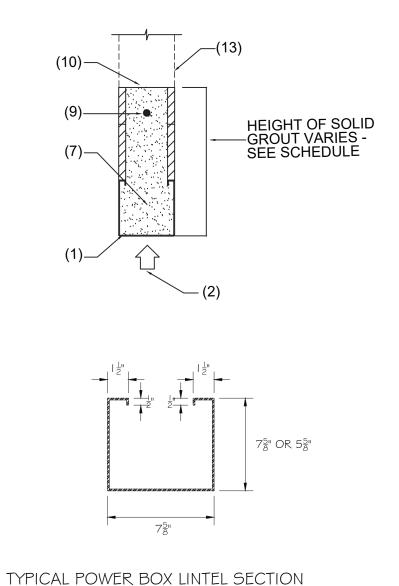
POWER LINTEL PSbox8 (7-5/8") 24" COMPOSITE

L-1	CLEAR		FILLED W/	FILLED W/
		LINTEL	#5 TOP	#5 T¢B
	SPAN	LENGTH	DETAIL "K"	DETAIL "KK"
1 0	1'-6"	2'-10"		
L-2	2'-2"	3'-6"		
L-3	2'-8"	4'-0"		
L-4	3'-2"	4'-6"	6746	6746
L-5	4'-0"	5'-4"	5305	5305
L-6	4'-6"	5'-10"	4696	4696
L-7	5'-2"	6'-6"	4068	4068
L-8	6'-2"	7'-6"	3380	3380
L-9	7'-0"	8'-4"	2957	2957
L-10	8'-0"	9'-4"	2566	2566
L-	9'-2"	10'-6"	2218	2218
L-12	10'-0"	11'-4"	2019	2019
L-13	11'-2"	12'-6"	1790	1790
L-14	12'-0"	13'-4"	1653	1653
L-15	12'-8"	14-0"	1557	1557
L-16	13'-4"	14'-8"	1471	1471
L-17	14'-0"	15'-4"	1393	1393
L-18	16'-0"	17'-4"	1197	1197
L-19	18'-0"	19'-4"	1201	1336
L-20	18'-8"	20'-0"	1104	1282
L-21	20'-8"	22'-0"	869	1037
L-22	22'-8"	24'-0"	693	832
L-23	24'-0"	25'-4"	625	918
L-24	26'-0"	27'-4"	507	757
L-25	28'-0"	29'-4"	413	629
L-26	30'-0"	31'-4"	337	525

POWER LINTEL PSbox8 (7-5/8") 24" COMPOSITE





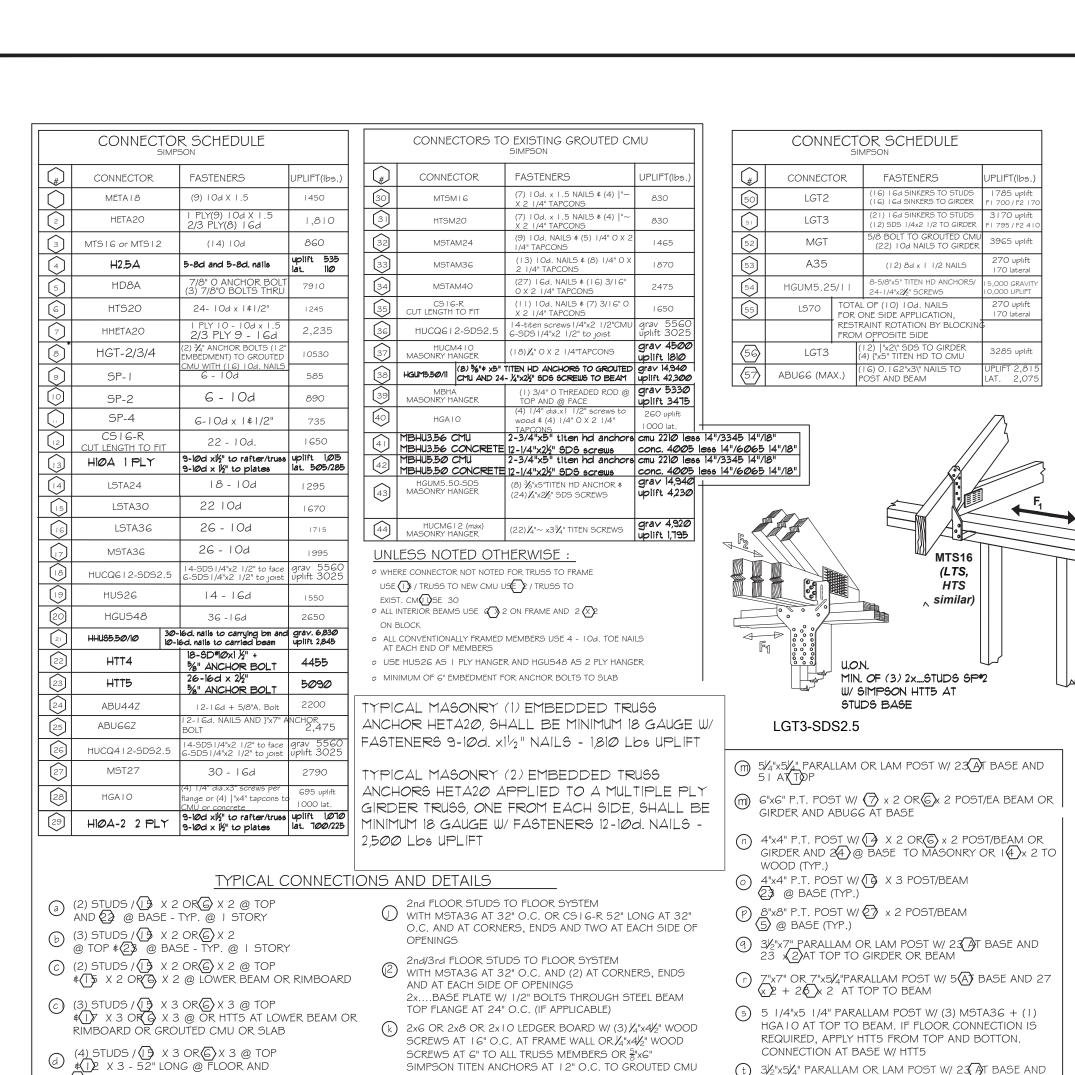


SILVESTRE **ENGINEERING** 465 MAITLAND AVE. ALTAMONTE SPRINGS, FL. 32701 PH: (321) 303-2954

EMAIL: daniel@silvestre-engineering ENGINEER

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REVISIONS



SIMPSON TITEN ANCHORS AT 12" O.C. TO GROUTED CMU

TO STUDS AND 12d. NAILS AT 12" O.C. TO PLATE OR 10d.

x 3" NAILS AT 6" TO ALL LAPPED TRUSS MEMBERS OR (3)

STAGGERED ROWS OF 12d. NAILS AT 24" O.C. TO BEAM

FOR CONNECTION TO GROUTED CMU APPLY (2) ROWS OF

HETA20 AT 24" O.C. TO GROUTED CMU and MSTA36 OR

OPTIONAL 2x4 TOP AND BOTTOM RIBBON W/ HETA20 AT 24" O.C. AND A35 AT BOTTOM 2x4 RIBBON TO EACH

CS16-R 52" LONG AT 32" O.C. TO 2nd FLOOR FRAME

CONT. I 3/4" x FLOOR DEPTH LVL RIMBOARD WITH

1/8" SIMPSON TITEN ANCHORS AT 18" O.C.-MIN. 5" EMBED

(k) 2x12 OR 2x10 LEDGER BOARD W/ (3) 12d. x 3 | " NAILS AT 16" O.C. AT FRAME WALL OR (2) 12d. NAILS AT 16" O.C.

OPTIONAL CONNECTIONS AT BASE:
TO TOP OF CMU W(5)/TO TOP OF FLOOR

TO TOP OF CMU W(5)/ TO TOP OF FLOOR BEAM OR GIRDER W/ 15 x 3 OR 6 3

(2) STUDS / ② + → @ TOP \$ HTT5 AT MASONRY OR → × 2 OR ⑥ × 2 @ BASE

BEAM OR GIRDER W/ 16 3 OR 6 3

(5) STUDS / (1) X 3 OR (6) X 3 @ TOP (1) 2 X 3 - 52" LONG @ FLOOR AND

(3) STUDS / (13) X 3 OR (6) X 3 @ TOP \$(1)6 X 3 @ FLOOR AND (5) @ BASE

(t) 3½"x5½" PARALLAM OR LAM POST W/ 23(A)T BASE AND COLUMN CAP CCQ46SDS2.5 WITH STRAPS ROTATED

(U) 3½"x3½" OR 3\"x5|" VERSA LAM POST SITS ON CMU W/

CONNECT TO 2 PLY GIRDER W/ LGT2 AND 3 PLY LGT3

(v) 4x6 POST WITH ABU46Z AT BASE AND (2) LSTA36 OR (2) MTS20 AT TOP TO BEAM OR GIRDER TRUSS

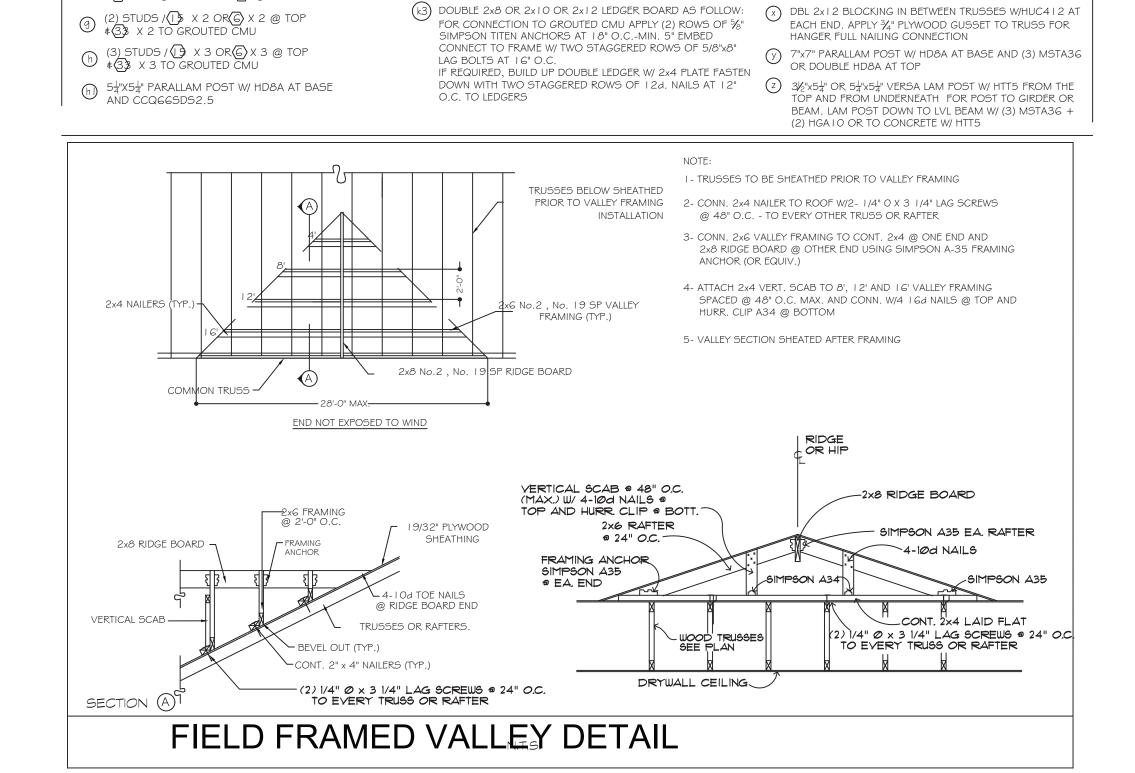
(w) 5¼"x5¼" OR 5¼"x7"PARALLAM OR LAM POST W/ 23(A)

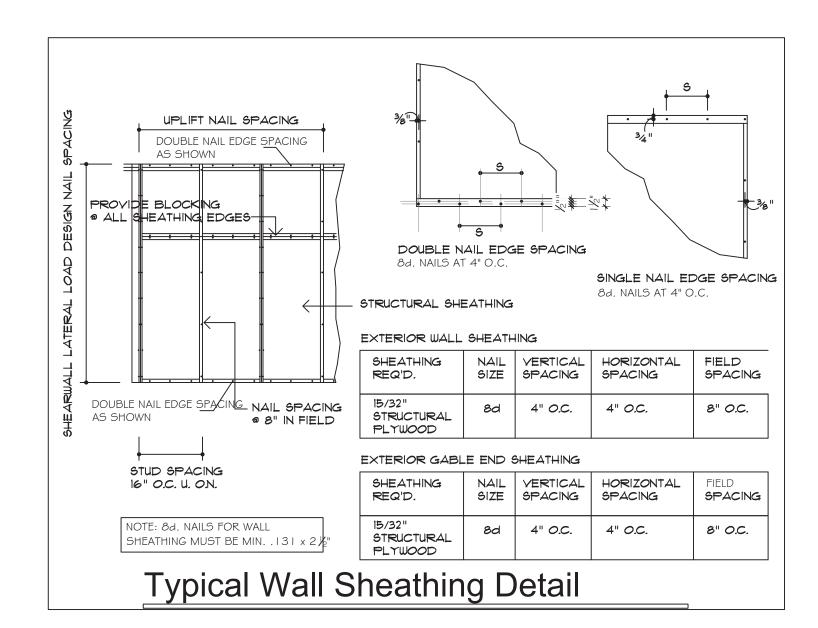
BASE AND 23 () AT TOP TO GIRDER OR BEAM

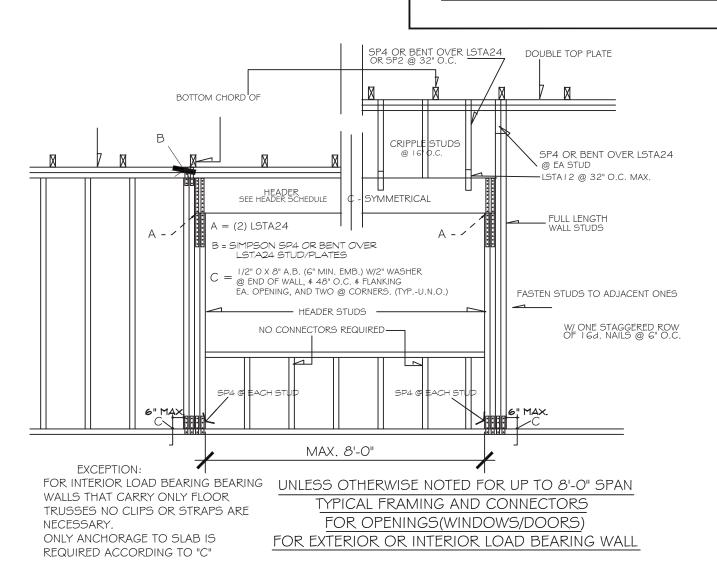
CAP TO BEAM

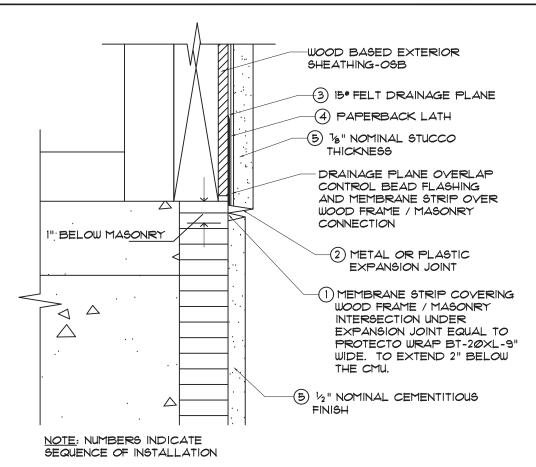
BASE TO GIRDER OR BEAM AND CONNECTION AT TOP

WITH LGT2 FROM FROM THE FRONT ONE HGA I O FROM

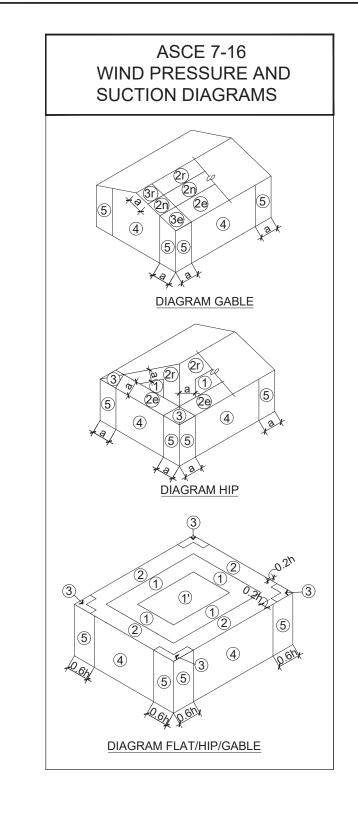








STUCCO FLASHING DETAIL NT. @ CMU / FRAME INTERFACE



ROOF NAILING SCHEDULE

NAILING ZONES SHINGLE, METAL AND TILE

Where the sheathing thickness is greater than 15/32 inches, sheathing

shall be fastened with ASTM F1667 RSRS-03 (2 $\frac{1}{2}$ " × 0.131") nails or

ASTM F1667 RSRS-04 (3" × 0.120") nails. RSRS-01, RSRS-03 and

ZONE (1') ----- : 6" O.C. EDGE AND 6" O.C. IN FIELD

ZONE (1)----- : 6" O.C. EDGE AND 6" O.C. IN FIELD

ZONES (2) AND (3): 4" O.C. EDGE AND 4" O.C. IN FIELD

ZONES(1) AND (2e): 6" O.C. EDGE AND 6" O.C. IN FIELD

ZONES (2n) AND (2r): 6" O.C. EDGE AND 6" O.C. IN FIELD

ZONES (3e) AND(3r): 4" O.C. EDGE AND 4" O.C. IN FIELD

ZONE (1)----- : 6" O.C. EDGE AND 6" O.C. IN FIELD

ZONE (2e) ----- : 6" O.C. EDGE AND 6" O.C. IN FIELD

ZONES (2r) AND (3e): 4" O.C. EDGE AND 4" O.C. IN FIELD

(I) EDGE SPACING ALSO APPLIES OVER GABLE END WALLS OR TRUSSES

12" SPACE CHANGES TO 8", 6" TO 4", AND 4" OR 3" CHANGES TO 2 1/2".

1/2" GYPSUM CEILING : Use 5d Nails @ 7" on center

SECOND FLOOR NAILING: 10d @6" O.C. Edges (glue \$ nail) @ 12" O.C @ Field

IN LIEU OF I Od. RING SHANK NAILS WITH REDUCED SPACING AS NOTED BELOW.

-2x STUD CONT. TO

-16d SPIKES @ 12

2-2×4'S

2-2×4'S

2-2×6'S

2-2x8'S

-16d SPIKES @ 12

WALLS

3-2×4'S

3-2×6'S

3-2x8'S

3-2×10'S

TOP PLATE

TOP PLATE

TOP PLATE

-HEADER-CONT.

GLUE & NAILED

HEADER SCHEDULE FOR 4" WALL

PENING WIDTH | BEARING WALL | NON-BEARING

SHEARWALL

2-2×12 W/ 1/2"

PLYWD. FLITCH

HEADER SCHEDULE FOR 6" WALL

DPENING WIDTH | BEARING WALL | NON-BEARING

SHEARWALL

3-14" LVL

3-14" LVL

TOP PLATE

-HEADER-CONT.

GLUE & NAILED

USING 1/2" PLYWD.

USING 1/2" PLYWD.

(2) PASLODE 3" X .099" DIAMETER POWER DRIVEN COATED SCREW NAILS MAY BE USED

SHEATHING

(3) I Od. NAILS PER BOARD EDGES & FIELD ALL ZONES

(2) I Od. NAILS PER BOARD EDGES & FIELD ALL ZONES

Roof Sheathing Layout

-RAFTER/TRUSS

BUILDING

BLOCKING @ 48" O.C. FIRST 2 BAYS

EACH END AND 72" SPACING ON

SHADED AREA. MEETS APA

USE HEADER SIZES ABOVE UNLESS

90 'FY' HORIZONTAL SHEAR

90 'FY' HORIZONTAL SHEAR

16E 'E' MODULES OF ELASTICITY

UPLIFT CONNECTION REQUIREMENT

AT POINTS 'A'(TOP AND BOTTOM OF

HEADER STUDS. UPLIFT CONNECTION

IS REQUIRED AT EACH END OF HEADER

AND AT BOTTOM OF HEADER STUDS IN

ADDITION TO CONNECTORS AT WALL

UNSUPPORTED WALL HEIGHT

OR LESS

GREATER

THAN 10'

16E 'E' MODULES OF ELASTICITY

OTHERWISE NOTED ON FRAMING PLAN

PRIMARY FRAMING (BEAMS,GIRDERS,ETC...)

JOIST, RAFTERS, LINTELS, ETC. WERE SIZED

WERE SIZED USING
1800 'FB' EXTREME FIBER IN BENDING(SINGLE)

1200 'FB' EXTREME FIBER IN BENDING(SINGLE)

MINIMUM WALL AND HEADER STUD REQUIREMENTS

*THE HEADER STUD SHALL NOT BE REQUIRED IF THE HEADER IS SUPPORTED BY A SUITABLE FRAMING ANCHOR

MAXIMUM HEADER SPAN(FEET)

6 9 12 15 18

SUPPORTING END OF HEADER

NUMBER OF FULL LENGTH STUE AT EACH END OF HEADER

WIDTH

RSRS-04 are ring shank nails meeting the specifications in ASTM

FOR 15/32" OR LESS SHEATHING, ASTM F1667

FLAT/HIP/GABLE ROOF 0 TO 7 DEGREE

RSRS-01 (2 $\frac{3}{8}$ " × 0.113") nails.

GABLE SYSTEMS

HIP SYSTEMS

1x12 / 1x10 BOARD

1x6 / 1x8 BOARD

2-2x STUDS-

THAN 5'-0"

0'-0" TO 3'-0"

3'-1" TO 5'-0"

5'-1" TO 7'-0"

7'-1" TO 9'-0"

2-2× STUDS-

0'-0" TO 3'-0"

3'-1" TO 5'-0"

5'-1" TO 7'-0"

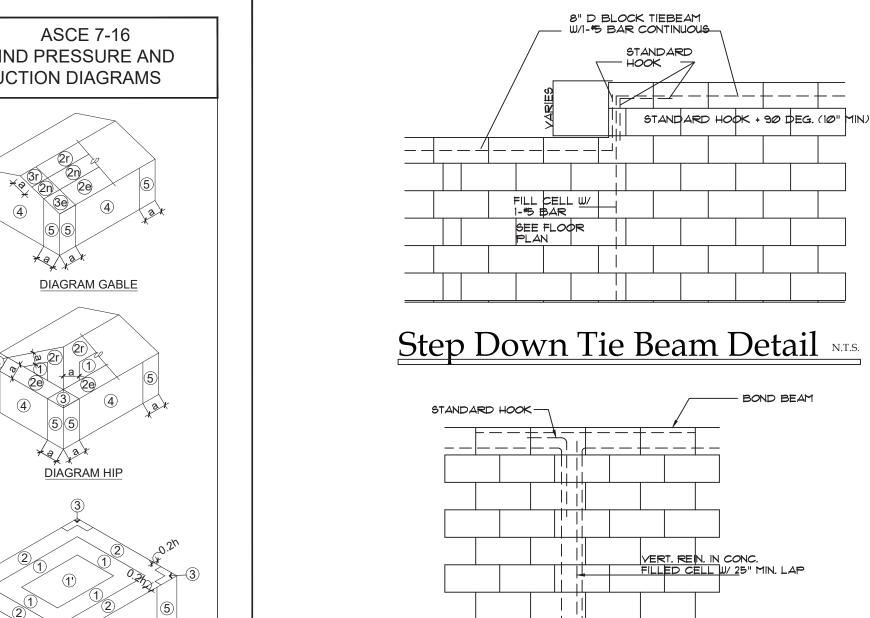
7'-1" TO 9'-0"

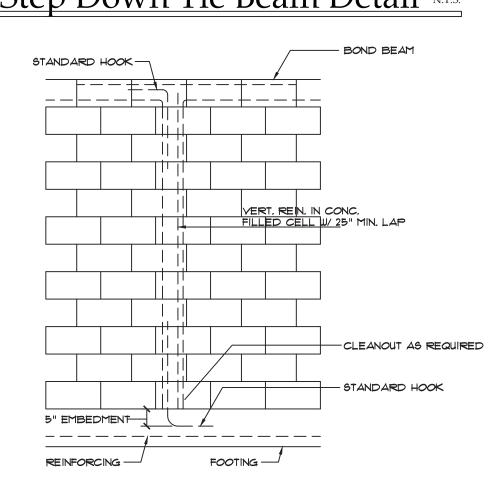
UNDER LINTELS W/

OPENINGS LESS THAN 5'-0"

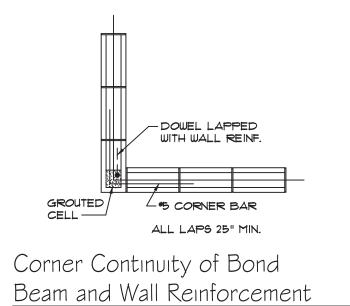
UNDER LINTELS W/

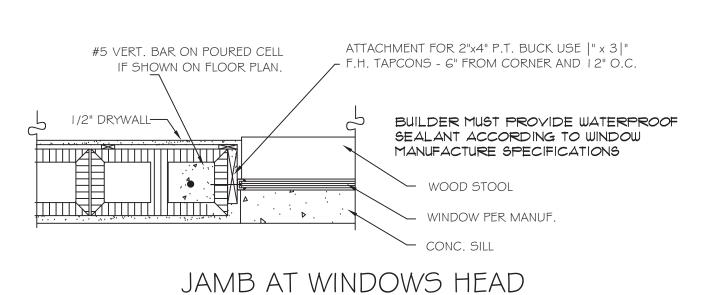
OPENINGS LESS

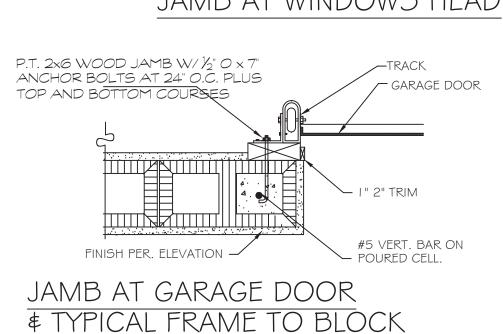


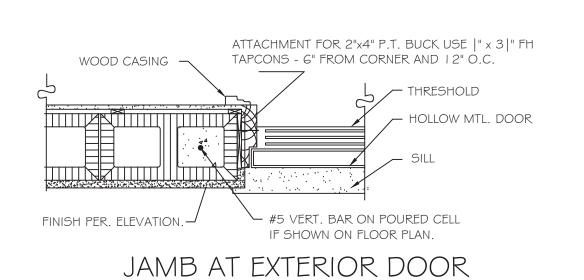


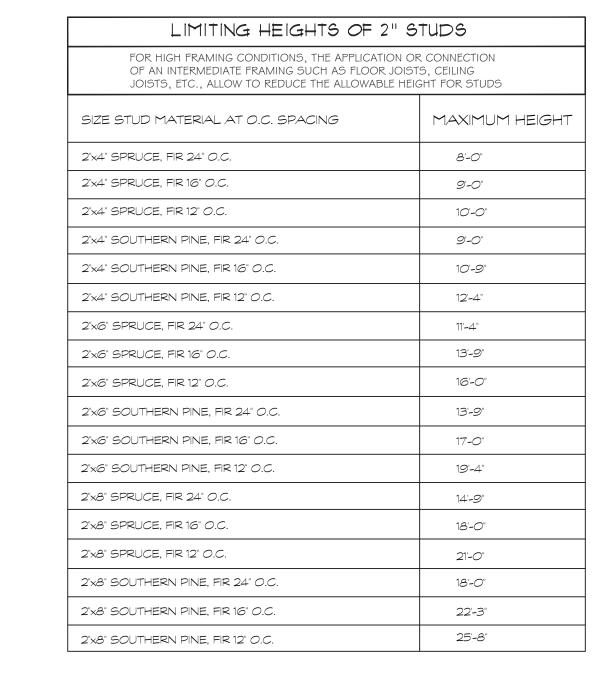
Typ. 1 Story Masonry Wall

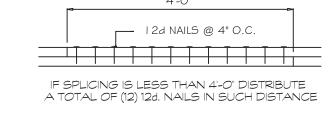




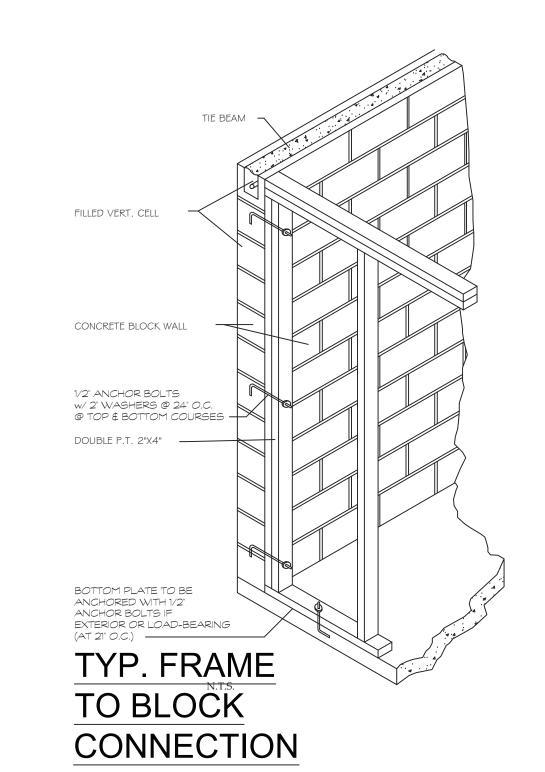








Top Plate Splice Detail





SILVESTRE **ENGINEERING** C.A. #30239 465 MAITLAND AVE.

ALTAMONTE SPRINGS, FL. 32701 PH: (321) 303-2954 MAIL: daniel@silvestre-engineering.

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SI

SCALE: NOTED

EVISIONS